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# 100-NR-2 Apatite Treatability Test FY09 Status: High-Concentration Calcium-Citrate-Phosphate Solution Injection for In Situ Strontium-90 Immobilization

## INTERIM REPORT

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December 2009



**Pacific Northwest**  
NATIONAL LABORATORY

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Pacific Northwest National Laboratory  
Richland, Washington 99352



## Summary

Following an evaluation of potential strontium-90 ( $^{90}\text{Sr}$ ) treatment technologies and their applicability under 100-NR-2 hydrogeologic conditions, the U.S. Department of Energy (DOE), Fluor Hanford, Inc. (now CH2M Hill Plateau Remediation Company [CHPRC]), Pacific Northwest National Laboratory, and the Washington State Department of Ecology agreed that the long-term strategy for groundwater remediation at the 100-N Area should include apatite sequestration as the primary treatment technology. This agreement was based on results from an evaluation of remedial alternatives that identified the apatite permeable reactive barrier (PRB) technology as the approach showing the greatest promise for reducing  $^{90}\text{Sr}$  flux to the Columbia River at a reasonable cost. This letter report documents work completed to date on development of a high-concentration amendment formulation and initial field-scale testing of this amendment solution.

The general approach for developing an in situ remedial technology for sequestration of  $^{90}\text{Sr}$  in groundwater through the formation of calcium-phosphate mineral phases (i.e., apatite) was documented in a project-specific treatability test plan, which provides a detailed discussion of test objectives and outlines the technical approach for development and deployment of the technology. Activities completed to date in support of the 100-NR-2 apatite treatability test that have been reported in previous documents include 1) laboratory-scale studies, 2) pilot-scale field testing with a low-concentration solution, 3) initial treatment of a 91-m (300-ft) -long PRB section with the low-concentration formulation, and 4) analysis of sediment samples collected following low-concentration treatment to determine whether any apatite had formed. A high-concentration amendment solution was formulated to maximize apatite formation within the targeted treatment zone while minimizing the short-term increases in  $^{90}\text{Sr}$  concentration associated with injection of high-ionic-strength solutions.

The original concept for field-scale deployment of the apatite PRB technology involved injection of a low-concentration, apatite-forming solution, followed by higher concentration injections as required to emplace sufficient treatment capacity to meet remedial objectives. The low-concentration injections were designed to provide a small amount of treatment capacity, thus stabilizing the  $^{90}\text{Sr}$  residing within the treatment zone, while minimizing  $^{90}\text{Sr}$  mobilization due to the injection of high-ionic-strength solutions. In theory, this approach would act to minimize  $^{90}\text{Sr}$  mobilization during subsequent high-concentration injections. However, results from the low-concentration field testing with a formulation containing stoichiometric calcium and phosphate concentrations for apatite precipitation, and subsequent laboratory studies aimed at optimizing the amendment formulation, determined that modifying the solution to a calcium-poor formulation was a better approach for maximizing apatite formation while minimizing short-term increases in  $^{90}\text{Sr}$  concentration. This modified formulation, which relies more heavily on calcium naturally present in the aquifer sediments as a source for apatite formation, was used during the high-concentration treatments documented in this report and will likely be used in all future PRB injection operations without low-concentration pretreatment.

In June and July 2008, high-concentration apatite solution injections were performed in support of the initial phase of barrier-emplacement operations (i.e., post-low-concentration pretreatment) for the 91-m (300-ft) -long apatite PRB section. Injection operations were conducted in 16 wells, the original 10 injection wells completed over the Hanford formation and upper contaminated portion of the Ringold Formation and 6 additional Ringold-only injection wells. Design criteria for the high-concentration injection operations were based on 1) amendment volume and mass injected, 2) amendment arrival at

adjacent wells, 3) water-level elevation during treatment, and 4) injection rate limitations associated with well plugging. An evaluation of compliance with these injection design criteria was used to assess operational performance and identify candidate wells for supplemental treatment. Injection design criteria were not fully met at 8 of the 16 injection well locations, with the primary deficiency at 4 of 8 locations being the limited vertical extent of Hanford formation treatment due to low-river-stage conditions during the injection. Wells whose extent of treatment did not meet design criteria should be considered for retreatment, or at a minimum, be placed on a watch list to identify premature  $^{90}\text{Sr}$  breakthrough in a timely manner.

Although injection design criteria were not fully met at a significant number of well locations, aqueous performance assessment monitoring data collected to date indicate good barrier performance. The average reduction in  $^{90}\text{Sr}$  concentrations at the four compliance monitoring locations was 95% relative to the high end of the baseline range and 84% relative to the low end of the baseline range, indicating that the performance objective specified in the treatability test plan (90% reduction in  $^{90}\text{Sr}$  concentration) was being met approximately 1 year after treatment. Consideration of these performance assessment data, in conjunction with the observed operational performance deficiencies, provide evidence that the apatite PRB technology may be relatively robust and able to perform effectively under the geohydrologic and geochemical heterogeneities present at field scale.

An evaluation based on sediment core samples that are planned for collection in November 2009, approximately 1 year after the high-concentration treatments, will be used to quantify the amount of apatite that has been formed from the barrier-emplacement operations completed to date. Based on these results, a determination will be made regarding the total volume of amendment solution, and thus the number of treatments, that will be required during full-scale deployment of the technology. If multiple high-concentration treatments are required, the timing of these injections must be considered. Development of the final injection approach for full-scale deployment of the PRB technology will consider the following design criteria: 1) emplacement of sufficient apatite to meet long-term remedial objectives, 2) limitation of permeability reduction by emplacing only the apatite content needed to meet remedial objectives, 3) limitation of short-term increases in  $^{90}\text{Sr}$  concentration associated with the injection of high-ionic-strength solutions, and 4) implementation cost.

## Acronyms and Abbreviations

ACS	American Chemical Society
°C	degree(s) Celsius
CERCLA	Comprehensive Environmental Response, Compensation, and Liability Act
CHPRC	CH2M Hill Plateau Remediation Company
Ci	curie(s)
cm <sup>2</sup>	square centimeter(s)
cm <sup>3</sup>	cubic centimeter(s)
DOE	U.S. Department of Energy
FH	Flour Hanford
ft	foot(feet)
FW	formula weight
FY	fiscal year
g	gram(s)
gpm	gallon(s) per minute
hr	hour(s)
kg	kilogram(s)
km	kilometer(s)
L	liter(s)
LWDF	liquid waste disposal facility
m	meter(s)
mg	milligram(s)
mg/L	milligram(s) per liter
mi	mile(s)
mM	millimolar
mmol	millimole(s)
OU	operable unit
pCi/L	picocurie(s) per liter
ppm	parts per million
PRB	permeable reactive barrier
PRD	Priest Rapids Dam
<sup>90</sup> Sr	strontium-90
SpC	specific conductance





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## 1.0 Introduction

Efforts to reduce the flux of strontium-90 ( $^{90}\text{Sr}$ ) to the Columbia River from past-practice liquid waste disposal sites have been underway since the early 1990s in the 100-N Area of the U.S. Department of Energy's (DOE's) Hanford Site in south-central Washington State (Figure 1.1 and Figure 1.2). Termination of all liquid discharges to the ground in 1993 was a major step toward meeting this goal. However,  $^{90}\text{Sr}$  adsorbed on aquifer solids beneath the liquid waste disposal sites and extending beneath the near-shore riverbed remains a continuing source to groundwater and the Columbia River. Researchers realized from the onset that the initial pump-and-treat system was unlikely to be an effective long-term solution because of the geochemical characteristics of  $^{90}\text{Sr}$ ; subsequent performance monitoring has substantiated this theory. Accordingly, the first *Comprehensive Environmental Response, Compensation, and Liability Act of 1980* (CERCLA) 5-year review re-emphasized the need to pursue alternative methods to reduce impacts on the Columbia River.

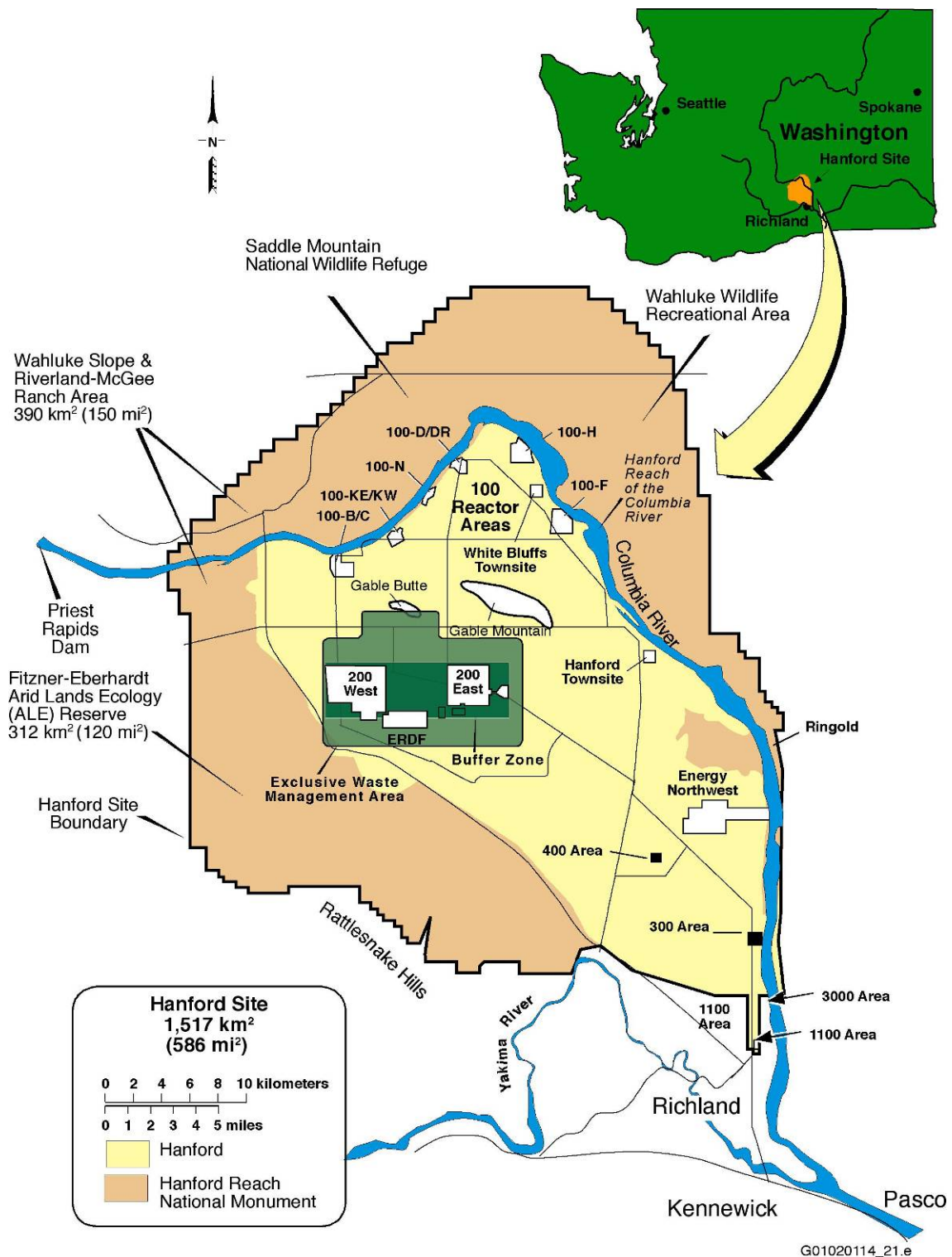
After an evaluation of potential  $^{90}\text{Sr}$  treatment technologies and their applicability under 100-NR-2 hydrogeologic conditions and presentation of the *Evaluation of  $^{90}\text{Sr}$  Treatment Technologies for the 100 NR-2 Groundwater Operable Unit*<sup>(a)</sup> at the December 8, 2004, public meeting, the U.S. Department of Energy (DOE), Fluor Hanford, Inc. (now CH2M Hill Plateau Remediation Company [CHPRC]), Pacific Northwest National Laboratory, and the Washington State Department of Ecology agreed that the long-term strategy for groundwater remediation at the 100-N Area should include apatite sequestration as the primary treatment technology. This agreement was based on results from an evaluation of remedial alternatives that identified the apatite permeable reactive barrier (PRB) technology as the approach showing the greatest promise for reducing  $^{90}\text{Sr}$  flux to the Columbia River at a reasonable cost. In July 2005, aqueous injection (i.e., the introduction of apatite-forming chemicals to the subsurface through standard injection wells) was endorsed as the interim remedy and selected for treatability testing.

The general approach for developing an in situ remedial technology for the sequestration of  $^{90}\text{Sr}$  in groundwater through the formation of calcium-phosphate mineral phases (i.e., apatite) was documented in a project-specific treatability test plan (DOE/RL 2006). This document includes a detailed discussion of test objectives and outlines the technical approach for development of the technology, up to and including field-scale deployment of a 91-m (300-ft) -long PRB section. Activities completed to date in support of the 100-NR-2 apatite treatability test that have been reported in previous documents include:

- Laboratory-scale studies were performed to 1) demonstrate in situ apatite formation and  $^{90}\text{Sr}$  sequestration proof-of-principle, 2) characterize the apatite formation and  $^{90}\text{Sr}$  sequestration mechanisms important to development of a pilot field-scale test design, and 3) optimize calcium-citrate-phosphate ( $\text{Ca-citrate-PO}_4$ ) amendment formulation to achieve site remedial objectives. Bench-scale development work is documented by Szecsody et al. (2007).

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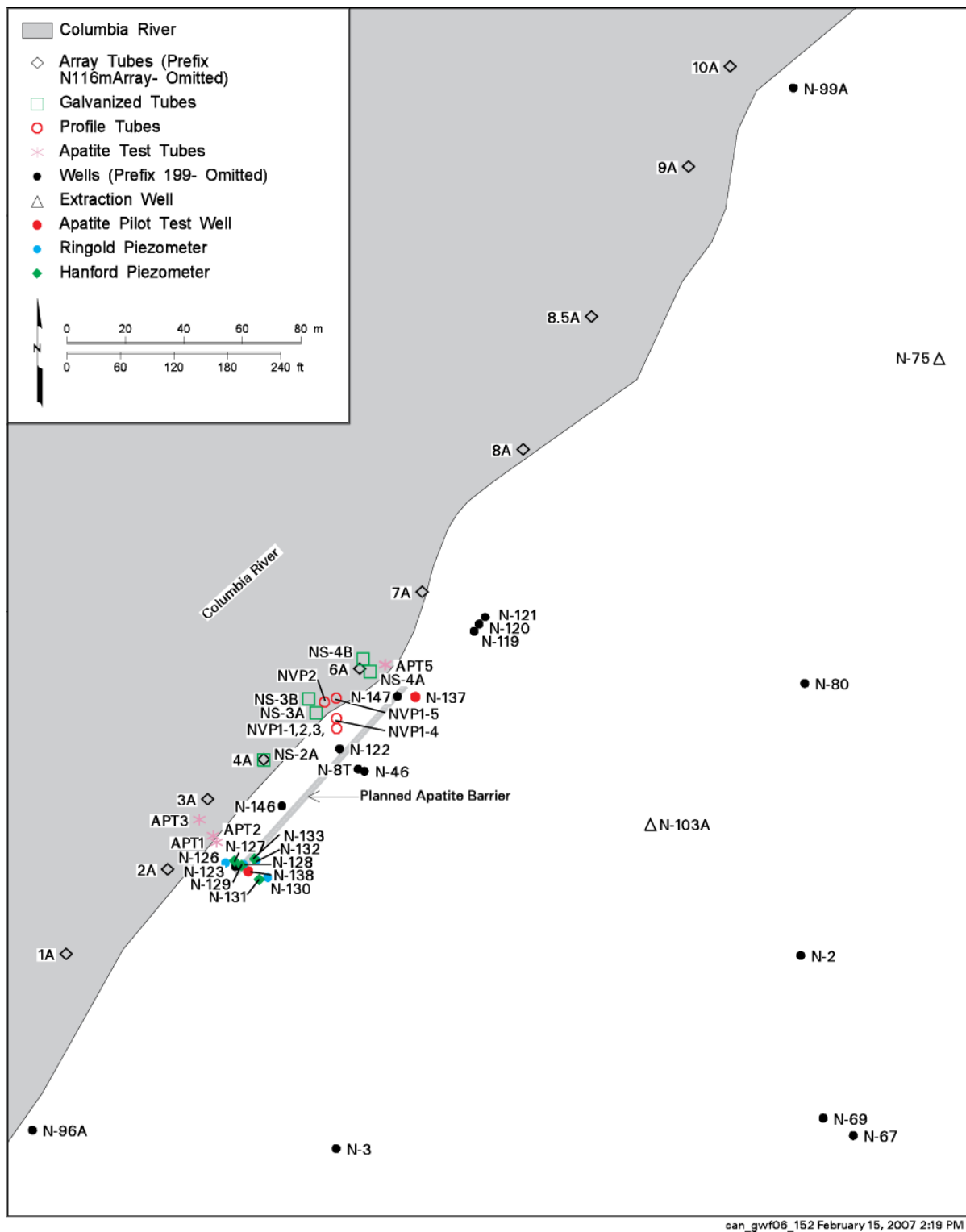
(a) Fluor Hanford, Inc. and CH2M HILL Hanford Group. 2004. *Evaluation of Strontium-90 Treatment Technologies for the 100-NR-2 Groundwater Operable Unit*. Letter Report available online at [http://www.washingtonclosure.com/projects/endstate/risk\\_library.html#narea](http://www.washingtonclosure.com/projects/endstate/risk_library.html#narea).



**Figure 1.1.** Index map for the Hanford Site in south-central Washington. The 100-N Area is located on the northern portion of the site along the Columbia River.



1.3



**Figure 1.3.** Aquifer tubes, seep wells, and monitoring wells on 100-N Area shoreline showing the location of the initial 300-ft-long apatite barrier section (from Hartman et al. 2007)



- Two pilot-scale field tests of the technology were conducted, one at each end of the 91-m (300-ft) -long PRB section, to characterize field-scale geohydrologic/geochemical conditions and assess the upscaling of laboratory results to actual field site conditions. Results from these pilot tests were used to refine the injection design for the remaining injections wells used to emplace the 91-m (300-ft) -long PRB section. The pilot-scale testing activities were reported by Williams et al. (2008).
- Initial treatment of the 91-m (300-ft) -long PRB section was performed using a low-concentration formulation and injection operations design that was based on results from the previous laboratory- and pilot-scale test results. The low-concentration Ca-citrate-PO<sub>4</sub> amendment formulation was designed to emplace a small amount of treatment capacity (i.e., apatite formation), while minimizing any short-term increase in <sup>90</sup>Sr concentration associated with injection of relatively high-ionic-strength solutions. A detailed description of the PRB-emplacement operations, including performance-assessment monitoring results through November 2007 is provided by Williams et al. (2008).
- Sediment core samples collected after the initial low-concentration treatments were analyzed for apatite content and compared with the apatite formation design target for this initial treatment. Although the apatite contents were small, they were sufficient to demonstrate that phosphate mineral phases had been formed, with the overlap zone between adjacent wells receiving an average treatment of 110% of the targeted apatite content within the Hanford formation and 30% treatment within the Ringold Formation (Szecsody et al. 2009). The Ringold apatite content data, in addition to amendment arrival responses observed in available Ringold Formation monitoring wells, support the decision to install Ringold-only injection wells that were used during subsequent high-concentration treatments (and will be used in all future injection operations).

This letter report documents work completed to date on the development of a high-concentration amendment formulation and initial field-scale testing of the amendment solution. The high-concentration amendment solution was formulated to maximize apatite formation within the targeted treatment zone, while minimizing the short-term increases in <sup>90</sup>Sr concentration associated with injection of high-ionic-strength solutions. Section 2.0 includes a brief discussion of pertinent background information for the site and the apatite PRB technology. It also describes the development and final formulation of the high-concentration Ca-citrate-PO<sub>4</sub> amendment solution. Section 3.0 provides a detailed description of the pilot-scale field testing and PRB injection operations that were performed using the high-concentration Ca-citrate-PO<sub>4</sub> amendment solution and Section 4.0 presents performance-assessment monitoring results collected through the end of fiscal year (FY) 2009, more than 1 year after injection of the high-concentration amendment. A summary of results and conclusions is presented in Section 5.0 and cited references are listed in Section 6.0. Appendixes A and B contain operational performance summary arrival curves for apatite amendment injections and performance plots for all groundwater monitoring points associated with the apatite barrier, respectively.



## 2.0 Background

The Hanford Site is a DOE-owned site located in southeastern Washington State near Richland, Washington (Figure 1.1). The 100-N Area is located along the Columbia River and includes the 100-N Reactor, a DOE nuclear reactor previously used for plutonium production.

Operation of the 100-N Area nuclear reactor required the disposal of bleed-and-feed cooling water from the reactor's primary cooling loop, the spent-fuel storage basins, and other reactor-related sources. Two crib and trench liquid waste disposal facilities (LWDFs) were constructed to receive these waste streams, and disposal consisted of percolation into the soil. The first LWDF (1301-N/116-N-1 shown in Figure 1.2 and Figure 2.1) was constructed in 1963, about 244 m (800 ft) from the Columbia River.

Liquid discharges to this LWDF contained radioactive fission and activation products, including  $^{60}\text{Co}$ ,  $^{137}\text{Cs}$ ,  $^{90}\text{Sr}$ , and tritium. Minor amounts of hazardous wastes such as sodium dichromate, phosphoric acid, lead, and cadmium were also part of the waste stream. When  $^{90}\text{Sr}$  was detected at the shoreline, a second crib and trench (1325-N LWDF/116-N-3) was constructed in 1983 farther inland and disposal at the first LWDF was terminated. Discharges to 1325-N stopped in 1991. The LWDFs have been excavated to remove the most highly contaminated soil and backfilled.

A more complete history of groundwater contamination at the 100-N Area is provided in *Hanford 100-N Area Remediation Options Evaluation Summary Report* (ITRD 2001). In summary, as a result of wastewater disposal practices, soils beneath the LWDF were contaminated from the surface sediments to the lower boundary of the unconfined aquifer. A portion of the contaminants migrated to the Columbia River via groundwater. To address contamination in the 100-N Area, the area was divided into two operable units (OUs). The 100-NR-1 OU contains all the source waste sites located within the main industrial area around the 100-N Reactor and the Hanford Generating Plant, and includes the LWDF surface sediments and shallow subsurface soil. The 100-NR-2 OU contains the contaminated groundwater and aquifer.

Hartman et al. (2007) described remediation activities in the 100-N Area related to the groundwater contamination as summarized below. As part of the source waste site remediation, contaminated soil was removed from 116-N-1 LWDF to a depth of ~4.6 m (15 ft) from 2002 to 2005 and was backfilled with clean soil in 2006. Contaminated soil was also excavated and removed from 116-N-3 LWDF (Figure 1.2) to a depth of ~4.6 m (15 ft) from 2000 to 2003 and backfilled with clean soil in 2004 and 2005. From 1995 to 2006, a groundwater pump-and-treat system for  $^{90}\text{Sr}$  was operated in the 100-N Area under a CERCLA interim action for the 100-NR-2 OU. This pump-and-treat system was put on cold standby in 2006 because it did not meet the remedial action objectives. DOE is testing alternative groundwater remediation methods for  $^{90}\text{Sr}$  in the 100-N Area, which includes the apatite PRB treatability testing described in this letter report.



**Figure 2.1.** Aerial photo of the treatability test site location in 2003. The 1301-N Crib has been backfilled since this photo was taken.

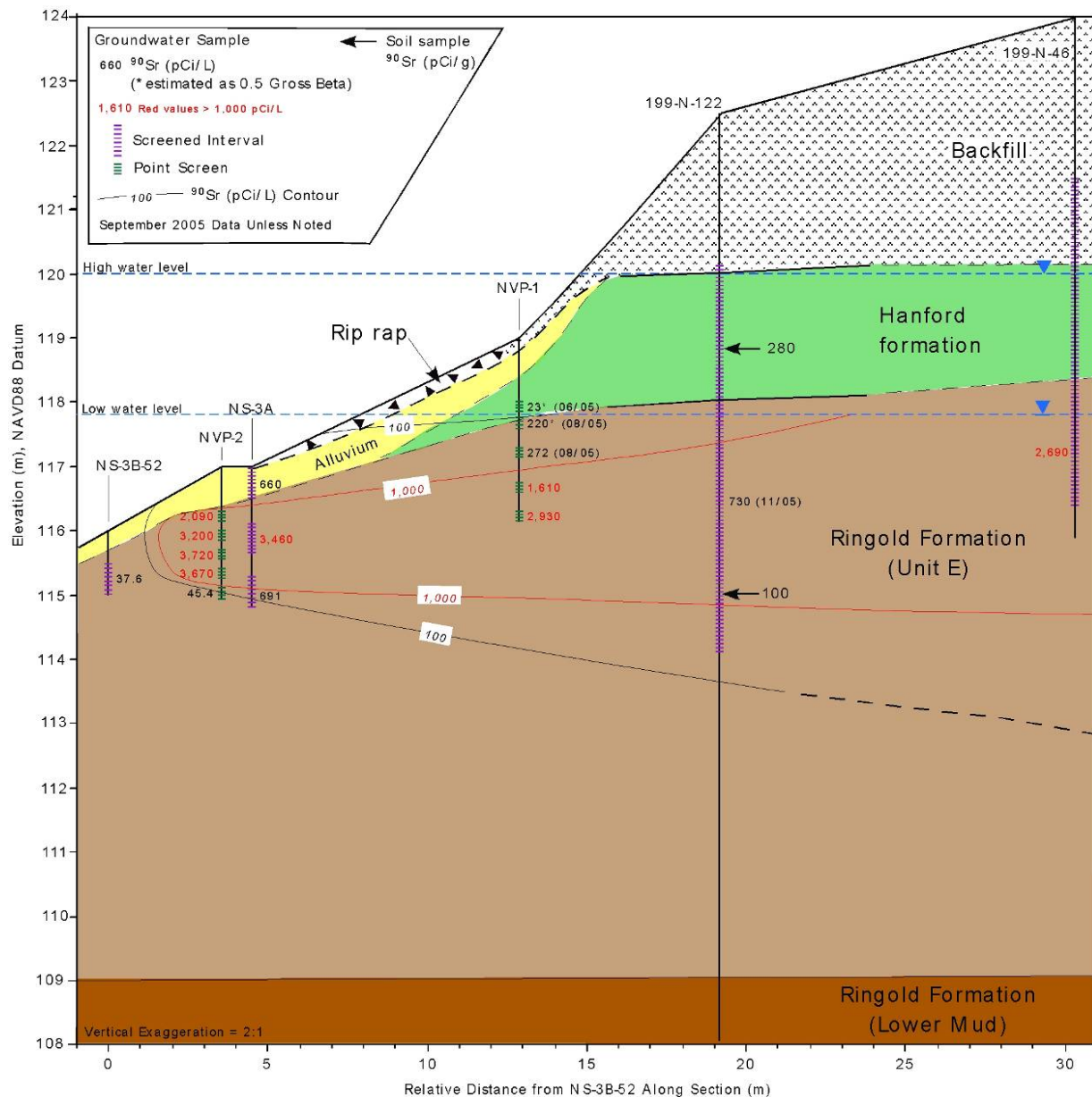
## 2.1 Site Hydrogeology

Stratigraphic units of significance at the 100-N Area include the following:

- Elephant Mountain Member of the Columbia River Basalt Group
- Ringold Formation
- Hanford formation.

The Elephant Mountain Member is an extensive basalt unit that underlies the fluvial-lacustrine deposits of the Ringold Formation and glaciofluvial deposits of the Hanford formation. The unconfined aquifer at the 100-N Area near the shoreline is composed of gravels and sands of the Hanford and Ringold formations, as shown in Figure 2.2. The Ringold Formation is composed of several lithologic facies; of most interest at the 100-N Area is Ringold Unit E, which forms the unconfined aquifer beneath the Hanford formation, and the Ringold Upper Mud Unit, which forms the base of the unconfined aquifer.

The uppermost stratigraphic unit in the 100-N Area is the Hanford formation, which consists of uncemented and clast-supported pebble, cobble, and boulder gravel with minor sand and silt interbeds. The matrix in the gravel is composed mostly of coarse-grained sand, and an open-framework texture is common. For most of the 100-N Area, the Hanford formation extends from the ground surface to just above the water table, 5.8 to 24.5 m (19 to 77 ft) in thickness. However, some channels of Hanford formation gravels extend below the water table.



**Figure 2.2.** 100-N Area site conceptual model in cross section

The uppermost Ringold stratum at the 100-N Area is Unit E, which consists of variably cemented pebble to cobble gravel with a fine- to coarse-grained sand matrix. Sand and silt interbeds may also be present. Unit E forms the unconfined aquifer in the 100-N Area and is approximately 12 to 15 m (39 to 49 ft) thick. The base of the aquifer is situated at the contact between Ringold Unit E and the underlying, much less transmissive, silty strata referred to locally as the Ringold Upper Mud, approximately 60 m (197 ft) thick.

The Hanford formation is much more transmissive than the underlying Ringold Unit E; however, due to geologic heterogeneity, the hydraulic conductivity in both units is highly variable. Typical values of 15.2 and 182 m/day (50 and 597 ft/day) have been used for modeling purposes for the Ringold and Hanford Units, respectively. Figure 2.2 depicts a cross section of the Hanford and upper Ringold Units in

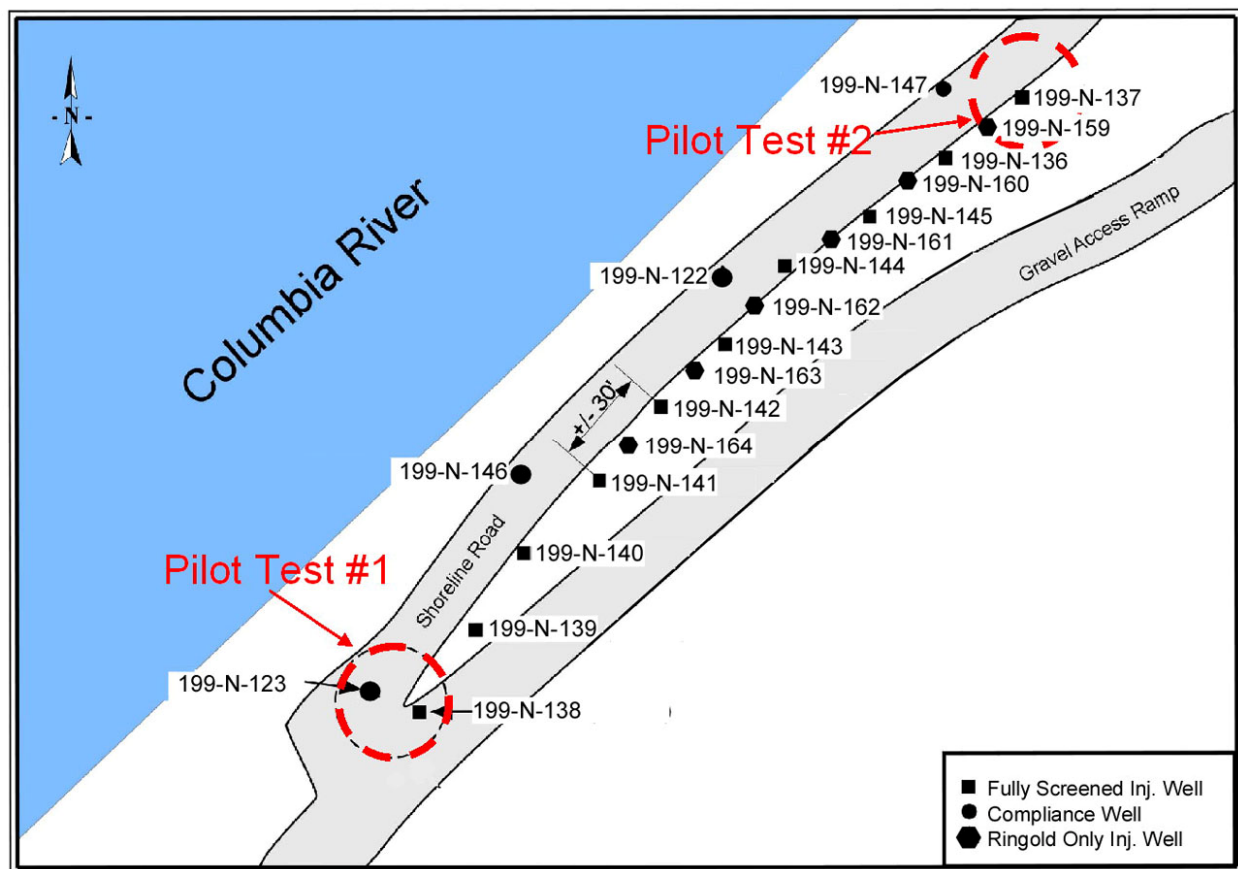
the near-river environment. As illustrated in Figure 2.2, the aquifer outcrops into the Columbia River channel and the high-river stage rises into the Hanford formation.

Site-specific hydrogeologic characterization data were gathered during the installation of treatability-test injection and monitoring wells. Two initial characterization wells were installed at the 100-N Area apatite treatability-test site in 2005 for detailed aquifer and sediment analysis, including depth-discrete  $^{90}\text{Sr}$  measurements of the sediment (wells 199-N-122 and 199-N-123; see Figure 2.3 and Figure 2.4). These wells, and two additional wells installed in 2006 (199-N-146 and 199-N-147) were designated as compliance monitoring wells. Also during 2006, 10 injection wells were installed at 9-m (30-ft) intervals along the 91-m (300-ft) PRB length. Two pilot-test sites located at the upstream and downstream ends of this PRB section (Figure 2.3, Figure 2.5, and Figure 2.6), which are equipped with extensive monitoring well networks, were used for the initial injections to develop the injection design for the remaining portions of the barrier. Conducting pilot tests at both ends of the barrier helped to assess differences in hydrogeologic conditions along the PRB test section. Comparison of test results from these two locations indicated that the permeability contrast between the Hanford and Ringold Formations was significantly less over the upstream most one third of the barrier. Williams and others (2008) estimated hydraulic conductivity for the Hanford and Ringold Formations over the upstream portion of the barrier at 12 and 10 m/day (39 and 33 ft/day), respectively. By contrast, hydraulic conductivity for the Hanford and Ringold Formations over the downstream portion of the barrier was estimated at 29 and 9 m/day (95 and 30 ft/day), respectively. It should be noted that these hydraulic conductivity estimates for the Hanford formation are significantly lower than had previously been used in 100-N Area modeling studies (Connelly 2001).

Geologic characterization data collected during well installation were used to develop a detailed geologic cross section along the 100-N Area shoreline. A southwest-to-northeast cross section through the treatability test site is presented in Figure 2.7. It should be noted that because the texture of the sediments between the upper stratigraphic units (Ringold Unit E, Hanford formation, and backfill) is so similar (i.e., sandy gravel), there is uncertainty associated with distinguishing between these units. Furthermore, the boundaries between these units are not discrete, but instead often grade into one another as a result of the sediment reworking and mixing during deposition. Although the actual Backfill-Hanford-Ringold contact depth remains somewhat uncertain, the geologic conceptual model depicted in Figure 2.7 represents the working model for the site.

Groundwater flows primarily in a north-northwesterly direction most of the year and discharges to the Columbia River, as shown in Figure 2.8, a local water table map constructed using April 2006 water-level data. The groundwater gradient varies from 0.0005 to 0.003. Near the LWDFs, average groundwater velocities are estimated to be between 0.03 and 0.6 m/day (0.1 and 2 ft/day), where 0.3 m/day (1 ft/day) is generally considered typical (DOE/RL 2006). However, groundwater flows near the river are significantly influenced by both diurnal and seasonal variability in Columbia River stage.

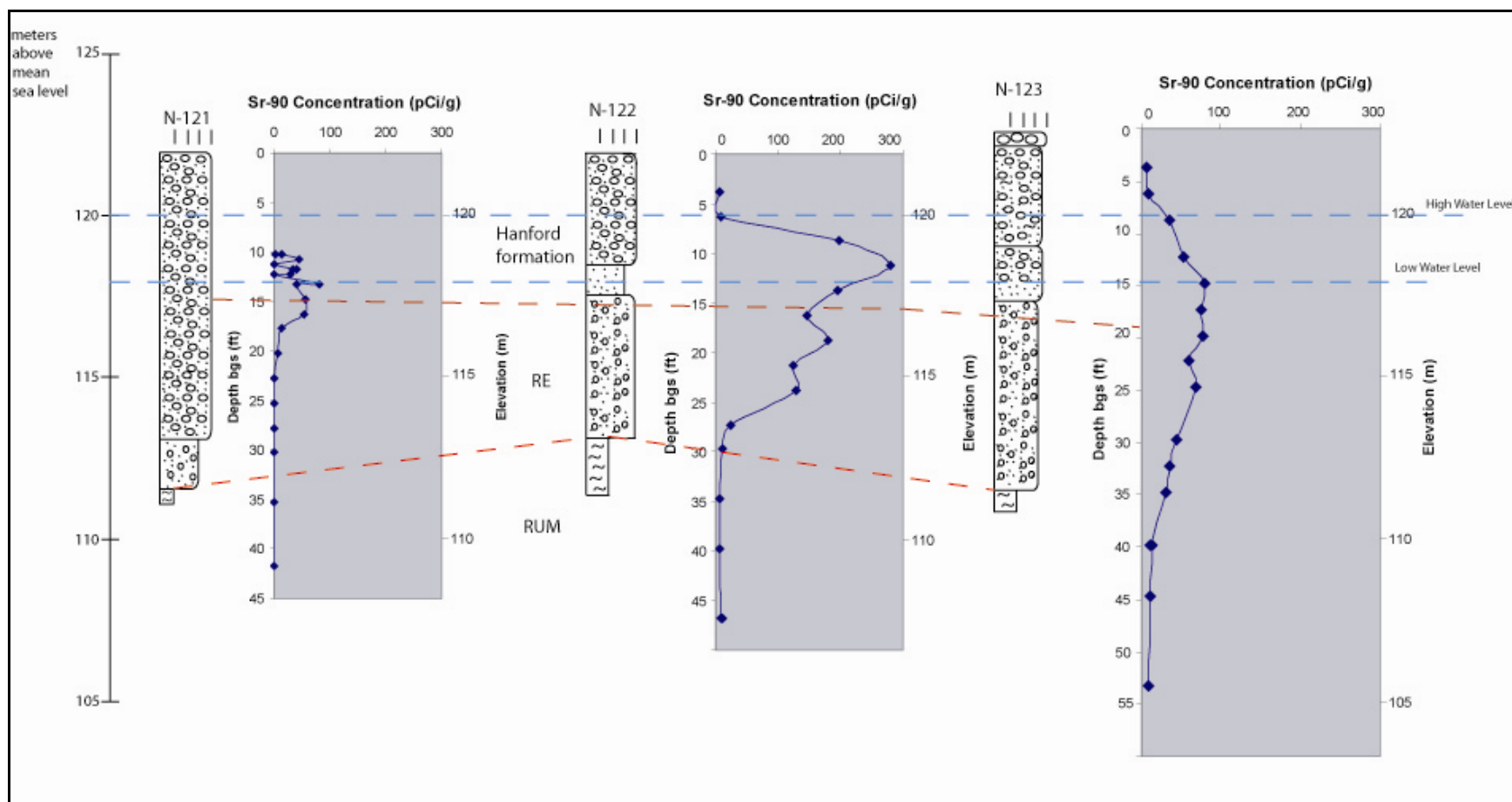
Fluctuations in river stage resulting from seasonal variations and daily operations of Priest Rapids Dam (PRD), located 29 km (18 mi) upstream of 100-N Area, have a significant effect on groundwater flow direction, hydraulic gradient, and groundwater levels near the river. The volume of water moving in and out of the unconfined aquifer on both a daily and seasonal basis is an order of magnitude greater than groundwater flowing as a result of the regional hydraulic gradient. In addition, with the changing direction of groundwater flow, pore-water velocities near the river may exceed 10 m/day (32.8 ft/day)



**Figure 2.3.** Map of the 100-N Area apatite treatability test plan site

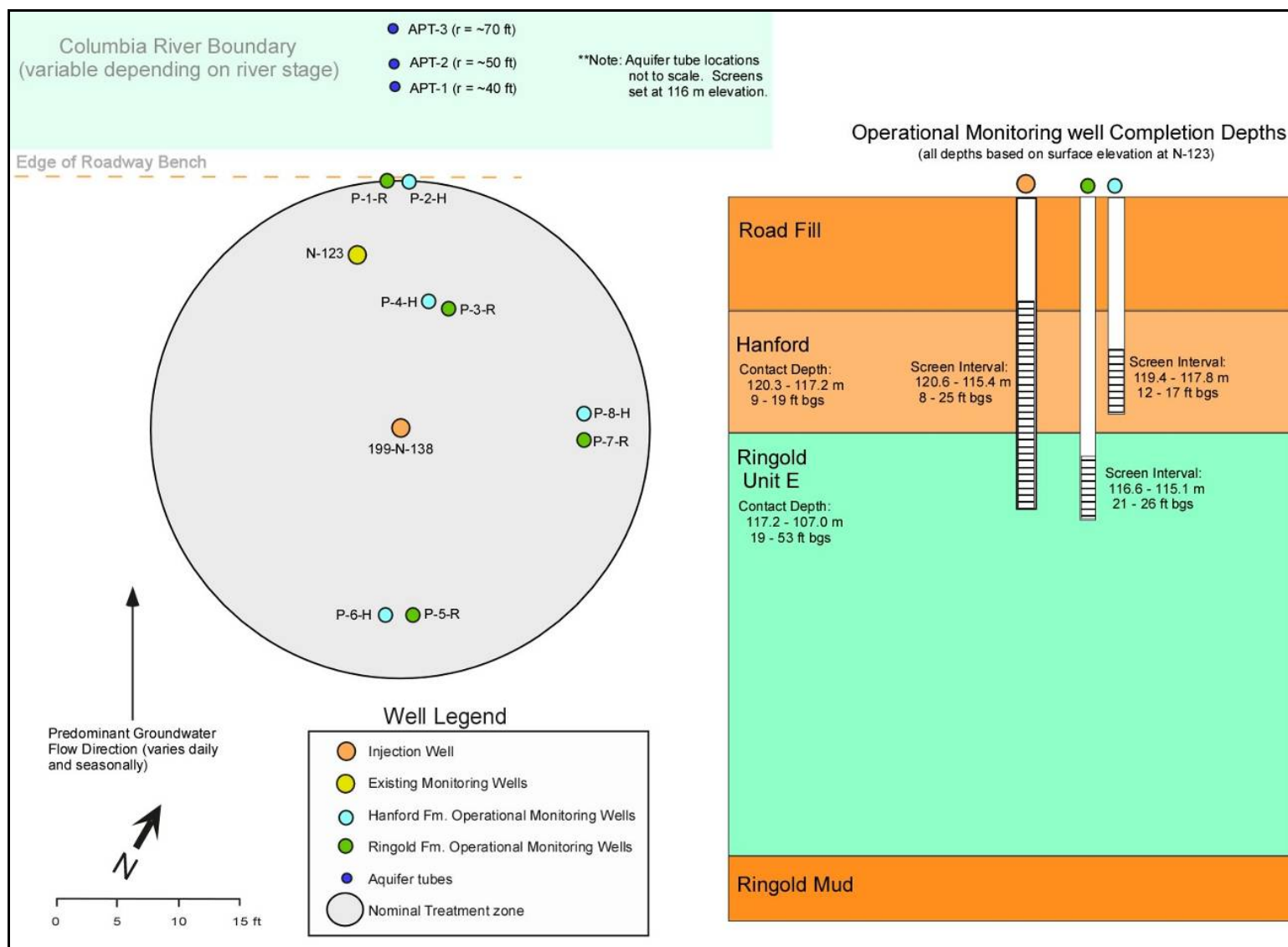
(Connelly 1999). During the high-river stage, river water moves into the bank and mixes with groundwater. The zone of mixing is restricted to within tens of meters of the shoreline. During low-river stage, this bank storage water drains back into the river and may be observed as springs along the riverbank. Springs, seeps, and subsurface discharge along the riverbank are the primary pathway of 100-N Area groundwater contaminants to the Columbia River. Additional details about the extent of seasonal and daily changes in river stage at the site from PRD discharge are reported by Williams et al. (2008).



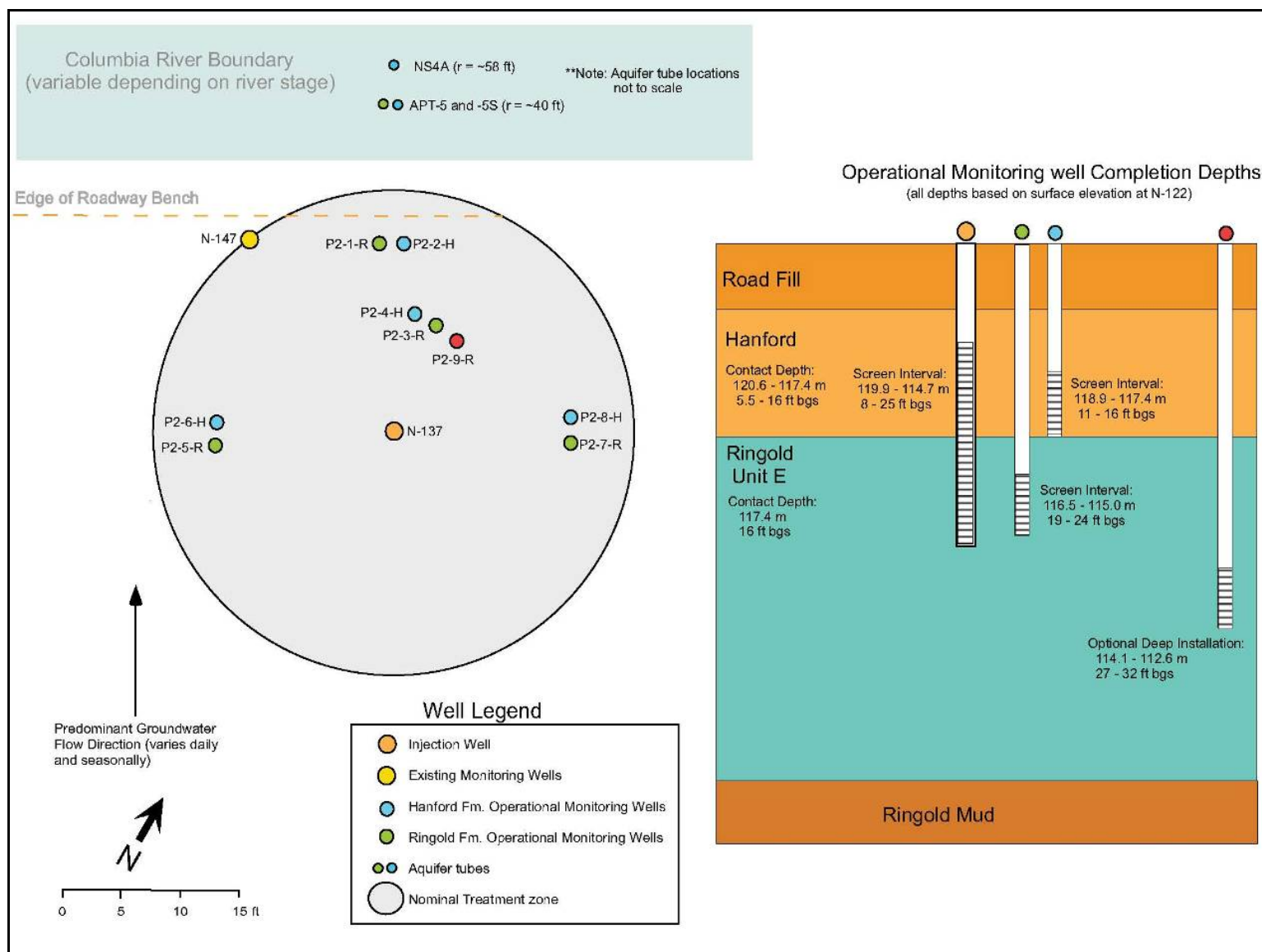


**Figure 2.4.**  $^{90}\text{Sr}$  profiles from three boreholes along the 100-N Area apatite treatability test site. See Figure 1.1 for borehole locations. Typical water level elevations range from approximately 118 to 120 m above mean sea level.

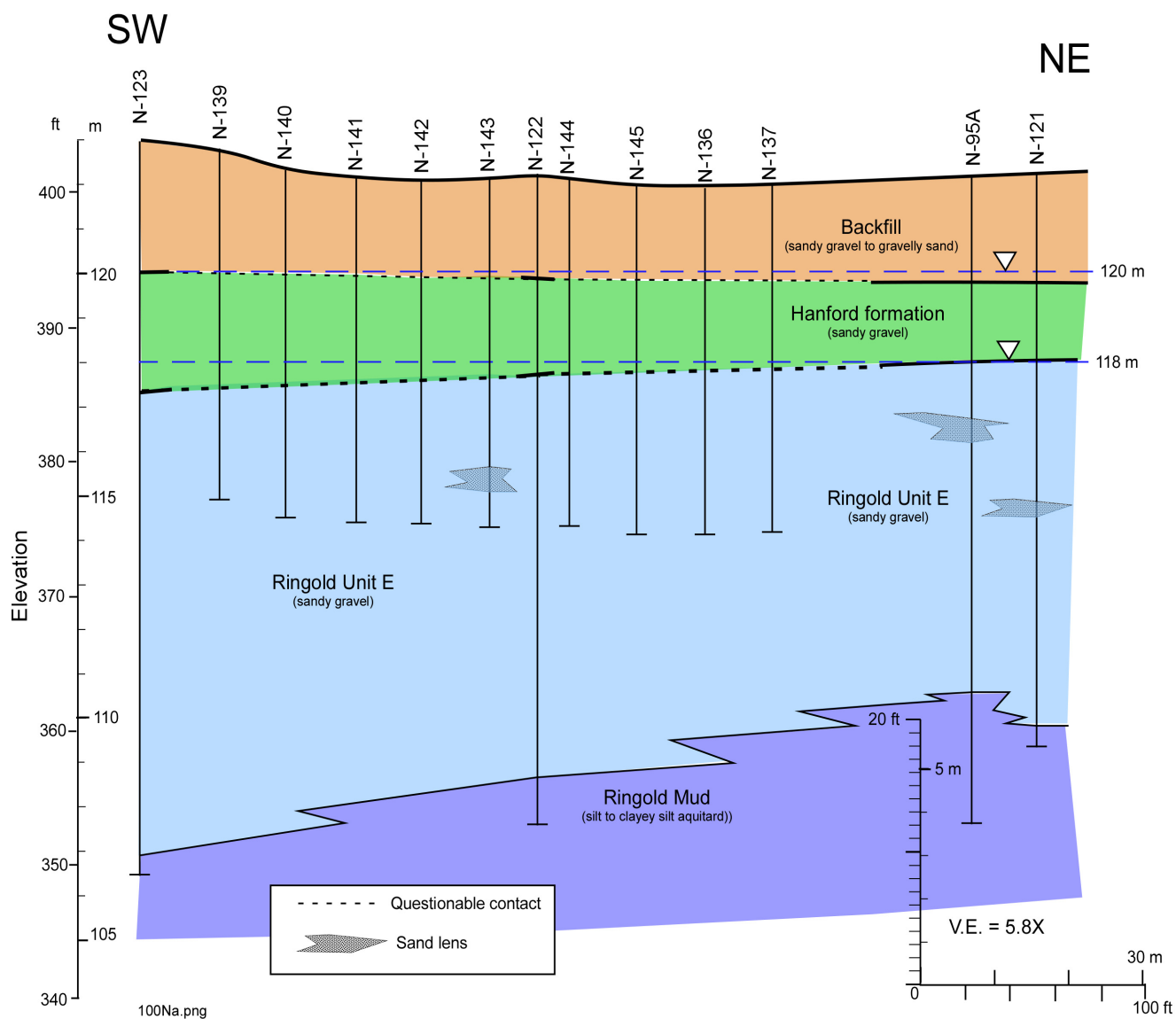




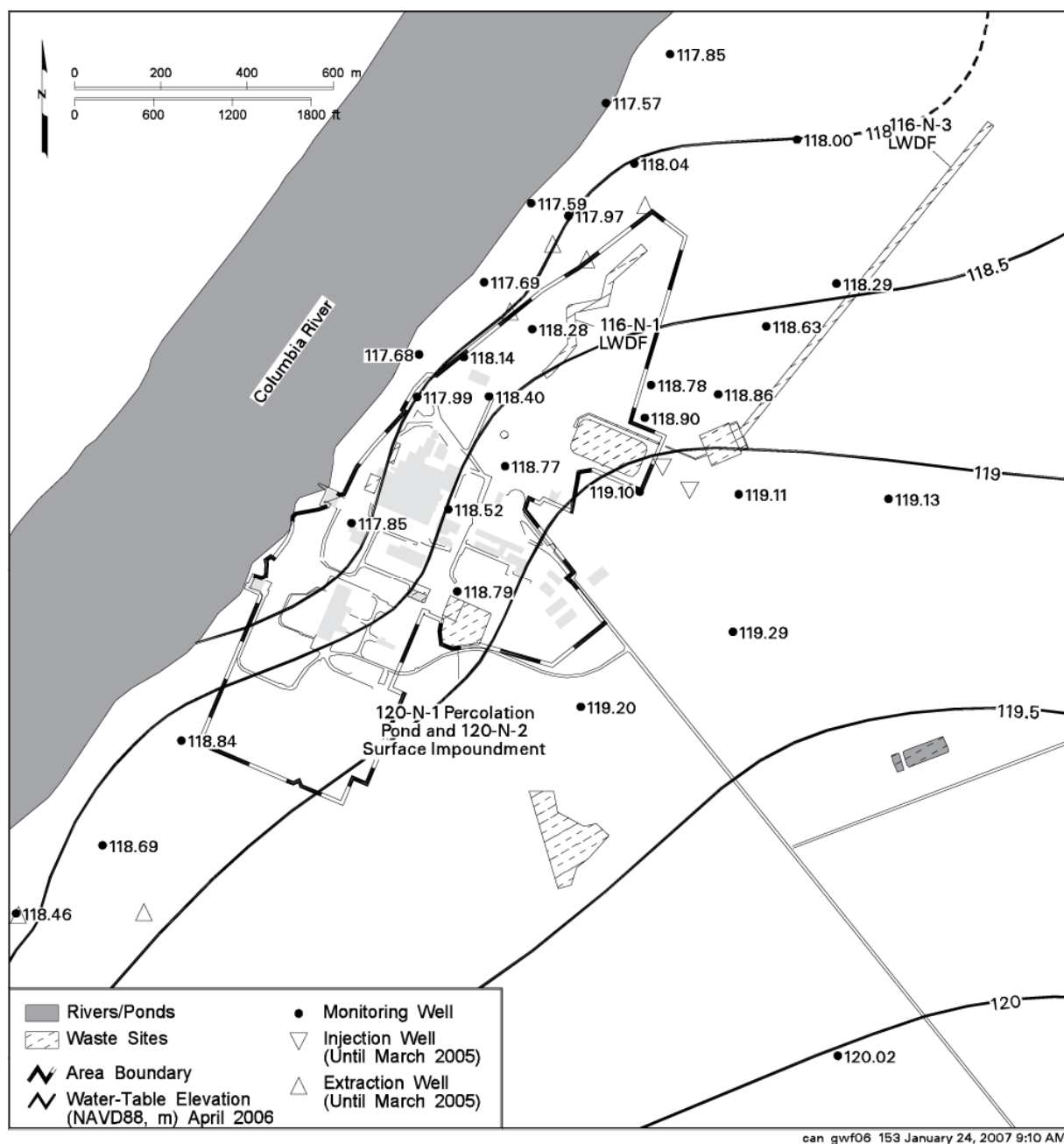
**Figure 2.5.** Pilot test site 1 (around well 199-N-138)



**Figure 2.6.** Pilot test site 2 (around well 199-N-137)



**Figure 2.7.** Geologic cross section updated based on data collected during installation of injection and compliance monitoring wells in 2006. Prefix 199- is omitted from well names.



**Figure 2.8.** Map of the 100-N Area water table, April 2006 (from Hartman et al. 2007)

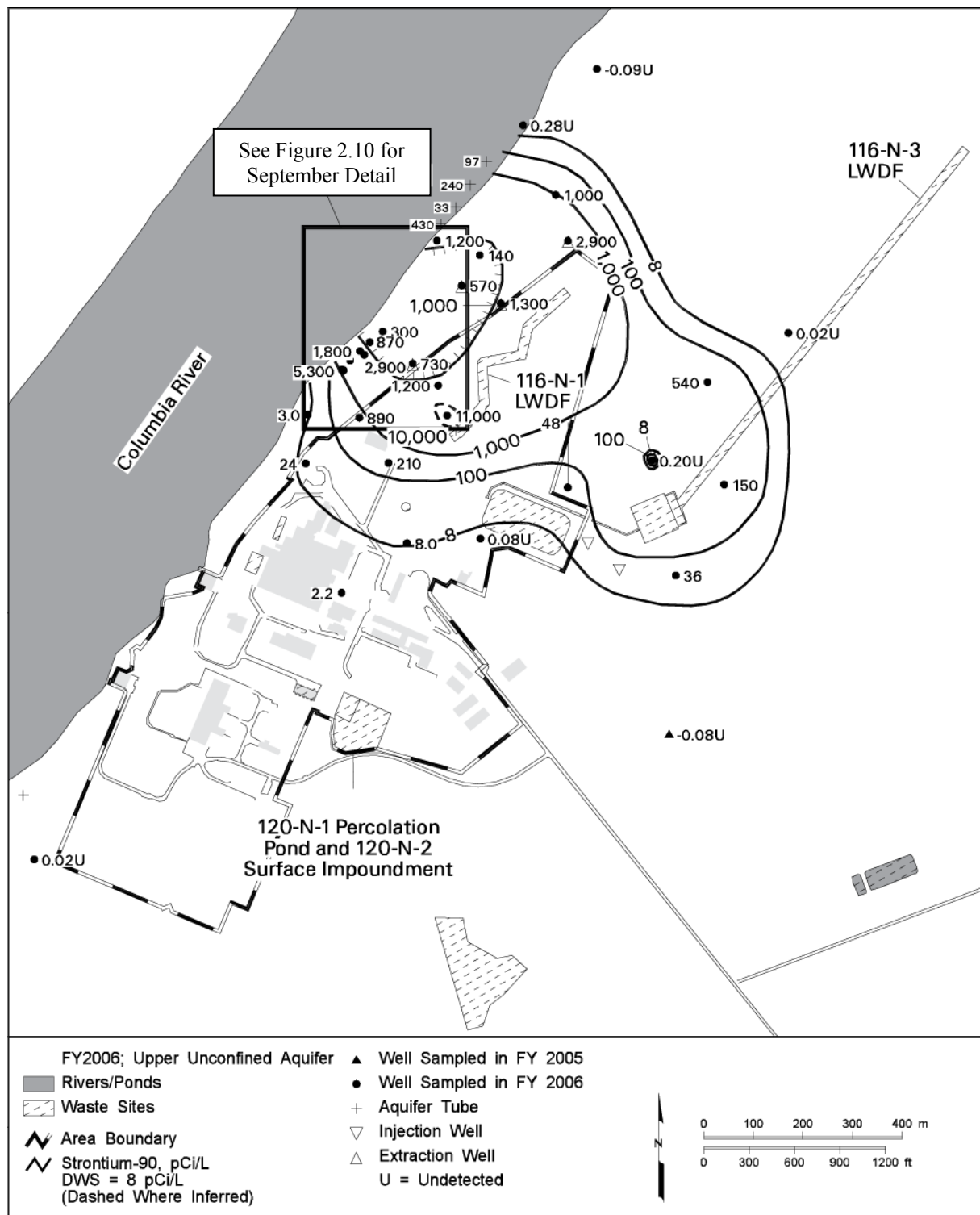
## 2.2 Nature and Extent of $^{90}\text{Sr}$ Contamination

Groundwater at the 100-N Area has been contaminated with various radionuclides and nonionic and ionic constituents. Contaminants of concern in the 100-NR-2 OU include  $^{90}\text{Sr}$ , tritium, nitrate, sulfate, petroleum hydrocarbons, manganese, and chromium (Hartman et al. 2007). Of primary concern is the presence of  $^{90}\text{Sr}$  in the groundwater and the discharge of  $^{90}\text{Sr}$  to the Columbia River via groundwater (Figure 2.9 and Figure 2.10). Although  $^{90}\text{Sr}$  sorbs to sediment strongly by ion exchange (retardation factor  $\sim 100$  in natural groundwater) and thus moves slowly under natural gradient conditions, higher ionic strength waste streams discharged to groundwater result in more rapid migration as  $^{90}\text{Sr}$  is exchanged off sediment surfaces. In addition, because of its chemical similarity with calcium,  $^{90}\text{Sr}$  bioaccumulates in plants and animals. With a half-life of 29.1 years, it will take approximately 300 years for the  $^{90}\text{Sr}$  concentrations present in the subsurface at 100-N Area to decay to below current drinking water standards.

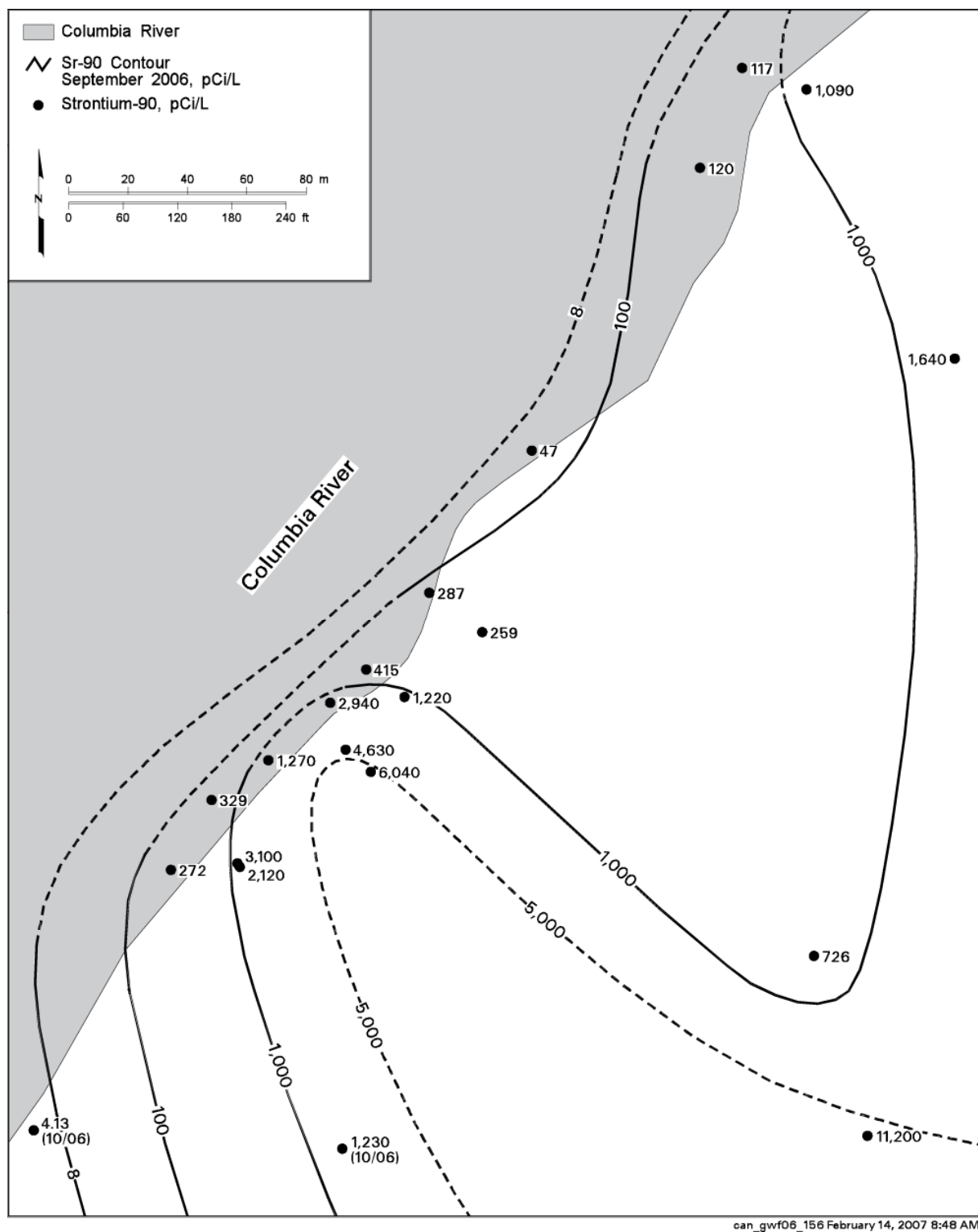
The zone of  $^{90}\text{Sr}$ -contaminated soils resulting from 30 years of wastewater discharge to the LWDFs includes the portions of the vadose zone that were saturated during discharge operations, and the underlying aquifer, which extends to the Columbia River (Figure 2.2). During operations, a groundwater mound approximately 6 m (20 ft) high was created. Not only was the water table raised into more transmissive Hanford Site sediments, but steeper hydraulic gradients were created, increasing the groundwater flow rate toward the river. While the 100-N Reactor was operating, riverbank seepage was pronounced. Since then, the number of springs and seeps has decreased in proportion to the decrease in artificial recharge caused by the wastewater disposal.

The majority of the 1500 curies (Ci) of  $^{90}\text{Sr}$  remaining in the unsaturated and saturated zones in the 100-N Area as of 2003 (DOE/RL 2004) is present in the vadose zone above the aquifer. An estimated 72 Ci of  $^{90}\text{Sr}$  are contained in the saturated zone, and approximately 0.8 Ci are in the groundwater (i.e.,  $^{90}\text{Sr}$  retardation factor  $\sim 100$ ). Data from soil borings collected along the riverbank indicate that  $^{90}\text{Sr}$  concentrations in soil reach a maximum near the mean water table elevation and then decrease with depth (BHI 1995) (see Figure 2.2 and Figure 2.4). This vertical contaminant distribution will also be reflected in depth-discrete groundwater concentration data. Because  $^{90}\text{Sr}$  has a much greater affinity for sediment than for water (high  $K_d$ ), its rate of transport in groundwater to the river is considerably slower than the actual groundwater flow rate. The relative velocity of  $^{90}\text{Sr}$  to groundwater is approximately 1:100. Under current conditions, approximately 0.14 to 0.19 Ci are released to the Columbia River from the 100-N Area annually (ITRD 2001).

In 1995, the  $^{90}\text{Sr}$  groundwater plume extended approximately 400 m (1300 ft) along the length of the Columbia River between the 1000 picocuries per liter (pCi/L) contours, and approximately 800 m (2600 ft) between the 8 pCi/L (drinking water standard) contours (Connelly 1999; ITRD 2001). The highest concentrations along the shoreline were observed between wells 199-N-94 and 199-N-46. An area of “preferential flow” was identified in the *Technical Reevaluation of the N-Springs Barrier Wall* (BHI 1995) that encompasses 199-N-94, 199-N-95, and 199-N-46. Because of an erosional feature in the Ringold Unit, the Hanford formation dips below the water table at this location, forming a more transmissive flow path between the disposal crib and the Columbia River (see Figure 2.2 and Figure 2.4).



**Figure 2.9.** Average  $^{90}\text{Sr}$  concentrations in 100-N Area, upper part of unconfined aquifer for September 2006 (from Hartman et al. 2007)



**Figure 2.10.**  $^{90}\text{Sr}$  distributions along 100-N Area shoreline, September 2006 (from Hartman et al. 2007)

N-Springs data from 1985 to 1991 show significantly higher concentrations of  $^{90}\text{Sr}$  in seep wells NS-2, NS-3, and NS-4 compared to the adjacent springs upstream and downstream (Figure 2.10) (BHI 1995). Well NS-3 and the neighboring monitoring wells 199-N-46 and 199-N-8T have currently and historically shown the highest  $^{90}\text{Sr}$  concentrations along the shoreline, with concentrations as high as 15,000 pCi/L observed at 199-N-46 (ITRD 2001; DOE/RL 2004). Recent clam data collected for the ecological risk assessment show the highest concentrations of  $^{90}\text{Sr}$  in clams were observed along the approximately 90 m (300 ft) of riverbank that encompasses well 199-N-123 and galvanized tube locations NS-2, NS-3 and NS-4 (Figure 1.3). The previous N-Springs, aquifer tube, groundwater, and clam data (DOE/RL 2006) all indicate that treating the 91 m (300 ft) of shoreline near well 199-N-46 will address the highest concentration portion, if not the majority, of the near-shore  $^{90}\text{Sr}$  contamination. The targeted length of shoreline is approximately between wells NS-1 and NS-4, as shown in Figure 1.3.

The  $^{90}\text{Sr}$  concentrations in groundwater along the Columbia River at the 100-N Area show significant temporal variability based on measurements from aquifer tubes and compliance monitoring wells installed prior to the apatite treatability test. Additionally, as discussed above there is a general spatial trend in  $^{90}\text{Sr}$  concentrations in the aquifer along the river, with the highest concentrations existing over the central and/or downstream portion of the 300-ft-long apatite PRB section, and concentrations decreasing from this high in both the upstream and downstream directions. Because of the short time between the installation of compliance, injection, and pilot-test monitoring wells at the 100-N Area apatite treatability test site and the Ca-citrate- $\text{PO}_4$  injections (started at the site in the spring of 2006), there were insufficient data from these wells to establish baseline conditions for  $^{90}\text{Sr}$ . Therefore, baseline  $^{90}\text{Sr}$  ranges were developed for the injection and compliance wells at the treatability test site based on gross beta concentrations from nearby aquifer tubes and limited pre-injection  $^{90}\text{Sr}$  monitoring from the treatability-test wells (Williams et al. 2008). These developed baseline ranges are shown on all post-treatment performance-assessment groundwater monitoring data plots for reference.

## 2.3 Field-Testing Approach

The original concept for field-scale deployment of the apatite PRB technology involved initial low-concentration, apatite-forming solution injections (Williams et al. 2008), followed by higher-concentration injections to emplace sufficient treatment capacity to meet remedial objectives. The low-concentration injections were designed to provide a small amount of treatment capacity, thus stabilizing the  $^{90}\text{Sr}$  residing within the treatment zone, while minimizing  $^{90}\text{Sr}$  mobilization due to the injection of high-ionic-strength solutions. In theory, this approach would act to minimize  $^{90}\text{Sr}$  mobilization during subsequent high-concentration injections. However, results from the low-concentration field testing with a formulation containing all the calcium and  $\text{PO}_4$  needed for apatite precipitation and subsequent laboratory studies aimed at optimizing the amendment formulation (Szecsody et al. 2007; see Section 2.5 of this letter report for a description of the high-concentration formulation) determined that modifying the solution to a calcium-poor formulation (i.e., less calcium in the injection solution) was a better approach for maximizing apatite formation while minimizing short-term increases in  $^{90}\text{Sr}$  concentration. This modified formulation, which relies more heavily on calcium naturally present in the aquifer sediments as a source for apatite formation, was used during the high-concentration injections and will likely be used in all future PRB injection operations without low-concentration pretreatment.

Injections at the treatability test site were timed during high- and low-river-stage periods to focus treatment in different portions of the contaminated zone. Initially, injection wells were screened across



both the Hanford formation and upper portion of the Ringold Formation. However, based on results from the first phase of field-scale injection testing at the pilot-test sites and remaining injection locations, wells screened only across the contaminated portion of the Ringold Formation were installed for better efficiency and treatment coverage. Injections conducted during high-river-stage periods targeted Hanford formation treatment as a result of the higher permeability of this formation relative the Ringold Formation. High-river-stage injections were scheduled in an attempt to take advantage of the highest possible river-stage conditions because contaminated sediments are present above the mean water table elevation (Figure 2.6). The contaminated upper portion of the Ringold Formation was targeted during low-river-stage periods to minimize reagent flux to the Hanford formation. Permeability contrast between the Hanford and Ringold formations was significantly less over the upstream portion of the barrier (injection wells 199-N-138, -139, -140, and -141), allowing for treatment of the entire screened interval (i.e., Hanford and Ringold inclusive) with a single injection operation at high-river stage.

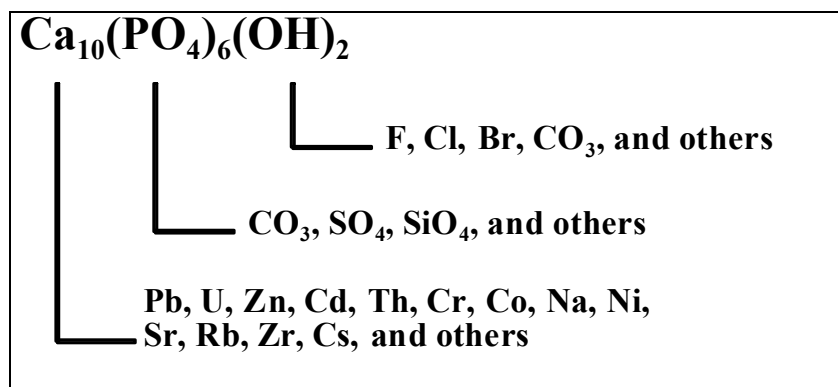
Analysis of the operational and early monitoring results of the pilot tests were used to modify the injection solution composition, injection volumes, and operational parameters. A tracer injection test and the first pilot apatite injection test (well 199-N-138) were conducted in the spring of 2006 during high-river-stage conditions. A second pilot test at a different well (199-N-137) at the downstream end of the barrier was conducted in September 2006 during low-river-stage conditions. Injections in the 10 barrier wells were conducted during two phases: the first in February-March 2007, which was supposed to target low-river-stage conditions but resulted in both low- and high-river-stage conditions, and a second phase in June-July of 2007 during high-river-stage conditions.

## 2.4 Treatment Technology Description

All technologies considered for  $^{90}\text{Sr}$  removal from groundwater at 100-NR-2 use apatite as the sequestering agent, differing only by emplacement method. This section describes apatite in general and the properties that make it a good sequestering agent, a description of the different forms of apatite commercially available that have been evaluated in bench-scale testing, and a description of the aqueous injection technology.

### 2.4.1 General Characteristics of Apatite

Apatite [ $\text{Ca}_{10}(\text{PO}_4)_6(\text{OH})_2$ ] is a natural calcium phosphate mineral occurring primarily in the Earth's crust as phosphate rock. It is also a primary component in the teeth and bones of animals. Apatite minerals sequester elements into their molecular structures via isomorphic substitution, whereby elements of similar physical and chemical characteristics replace calcium, phosphate, or hydroxide in the hexagonal crystal structure (Hughes et al. 1989; Spence and Shi 2005). Apatite has been used for remediation of other metals including uranium (Arey et al. 1999; Fuller et al. 2002, 2003; Jeanjean et al. 1995), lead (Bailliez et al. 2004; Mavropoulos et al. 2002; Ma et al. 1995), plutonium (Moore et al. 2005), and neptunium (Moore et al. 2003). Because of the extensive substitution into the general apatite structure (Figure 2.11), over 350 apatite minerals have been identified (Moelo et al. 2000). Strontium incorporation into apatite has also been previously studied (Smiciklas et al. 2005; Rendon-Angeles et al. 2000). Apatite minerals are very stable and practically insoluble in water (Tofe 1998; Wright 1990; Wright et al. 2004). The solubility product of hydroxyapatite is about  $10^{-44}$ , while quartz crystal, which is considered the most stable mineral in the weathering environment, has a solubility product ( $K_{sp}$ ) of  $10^{-4}$  (Geochem Software 1994). Strontiapatite,  $\text{Sr}_{10}(\text{PO}_4)_6(\text{OH})_2$ , which is formed by the complete substitution



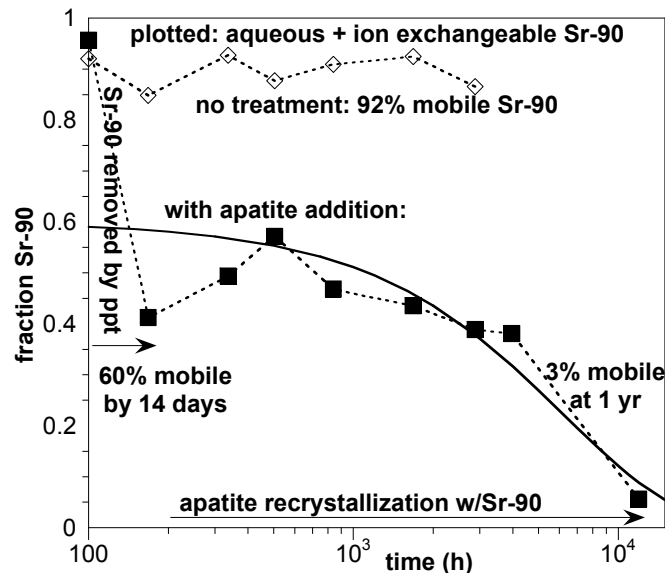
**Figure 2.11.** Cation and anion substitution in apatite

of calcium by strontium (or <sup>90</sup>Sr), has a  $K_{sp}$  of about  $10^{-51}$ , another  $10^7$  times less soluble than hydroxyapatite (Verbeeck et al. 1977). The substitution of strontium for calcium in the crystal structure is thermodynamically favorable and will proceed if the two elements coexist. Strontium substitution in natural apatites is as high as 11%, although dependent on available strontium (Belousova et al. 2002). Synthetic apatites have been made with up to 40% strontium substitution for calcium (Heslop et al. 2005). The mechanism (solid-state ion exchange) of strontium substitution for calcium in the apatite structure has been previously studied at elevated temperatures (Rendon-Angeles et al. 2000), and low-temperature aqueous rates under Hanford Site groundwater conditions (i.e., calcium/strontium ratio of 220/1) have also been studied (Szecsody et al. 2007, 2009).

Apatite can remove soluble strontium and <sup>90</sup>Sr from groundwater both during and after its formation. Removal can occur via precipitation of strontium in solution with PO<sub>4</sub> anion (Figure 2.12; <300 hours), adsorption to the apatite surface (adsorption is ~55 times stronger than to Hanford sediment), and slow substitution into the apatite structure (months to years time scale). Precipitation directly from solution, or homogeneous nucleation, generally occurs only at very high metal concentrations; that is, greater than 10 parts per million (ppm). However, apatite will act as a seed crystal for the precipitation of metal phosphates at much lower concentrations (Ma et al. 1995). The apatite itself serves as a small but sufficient source of phosphate to solution, and with low concentrations of cations such as strontium or calcium, heterogeneous nucleation occurs on the surface of the apatite seed crystal (Lower et al. 1998). Over time, the precipitated metals are sequestered into the apatite crystal matrix.

Although the rate of metal incorporation into the apatite crystal lattice can be relatively slow (on the order of months to years), the precipitation reaction is nearly instantaneous on the molecular scale. Initially, the precipitate formed is amorphous apatite; however, over time it will transform into a more stable apatite crystal.

Note that stable strontium and other competing cations in groundwater, especially the divalent transition metals (e.g., cadmium, zinc, iron, lead, manganese, etc.), can also be incorporated in the apatite structure. The average concentrations of stable strontium and competing cations present in groundwater will dictate the mass of apatite needed for long-term sequestration. Recent experiments measuring strontium incorporation in apatite from a solution containing only calcium and strontium to groundwater (containing all transition metals) found no difference in the strontium uptake mass (Szecsody et al. 2007).



**Figure 2.12.**  $^{90}\text{Sr}$  aqueous and ion exchangeable fraction in 100-N Area sediments with no apatite addition (diamonds) and with Ca-citrate- $\text{PO}_4$  solution addition (squares) to form apatite

The effect of competing cation concentrations is to reduce the in situ apatite longevity for a given mass loading. To achieve a desired longevity (e.g., a 300-year period during which most of the  $^{90}\text{Sr}$  will have decayed), loading must be increased to account for the competing cation effect.

## 2.4.2 Apatite Placement in the Subsurface

Various emplacement technologies have been considered for testing and deployment at the Hanford 100-N Area (DOE/RL 2006). Vertical hydrofracturing and pneumatic injection of solid-phase apatite are potential approaches for emplacement of solid mineral apatite particles into the subsurface. Excavation of the riverbank for trench-and-fill emplacement of solid-phase apatite is another option. In contrast, Ca-citrate- $\text{PO}_4$  solution injections form apatite through in situ precipitation reactions between chemical precursors in aqueous form. The advantages of aqueous injection are 1) the potential to create a larger treatment zone surrounding the point of injection than the other technologies, 2) some  $^{90}\text{Sr}$  is quickly incorporated into apatite during precipitation (Figure 2.12), and 3) minimal disturbance of the subsurface during apatite emplacement. Although each technology has advantages and disadvantages, the Ca-citrate- $\text{PO}_4$  injection technology was chosen because it provides the most economic emplacement methodology to treat  $^{90}\text{Sr}$  in the near-shore sediments. A weakness of all of these apatite technologies is that the  $^{90}\text{Sr}$  is not removed from the sediment until radioactive decay occurs, as the  $^{90}\text{Sr}$  is incorporated into the apatite crystalline structure.

The method selected to emplace apatite in subsurface sediments at the 100-N Area is to inject an aqueous solution containing a Ca-citrate complex and Na-phosphate. Citrate is needed to keep calcium in solution long enough (days) to inject into the subsurface, because a solution containing  $\text{Ca}^{2+}$  and phosphate only will rapidly form mono- and di-calcium phosphate, but not apatite (Andronescu et al. 2002; Elliot et al. 1973; Papargyris et al. 2002). Relatively slow biodegradation of the Ca-citrate complex (days) allows sufficient time for injection and transport of the reagents to the areas of the aquifer where treatment is required. As Ca-citrate is degraded (Van der Houwen and Valsami-Jones 2001; Misra 1998),

the free calcium and phosphate combine to form amorphous apatite. The formation of amorphous apatite occurs within a week and crystalline apatite forms within a few weeks. Citrate biodegradation rates in Hanford 100-N Area sediments (water saturated) at temperatures from 10°C to 21°C (aquifer temperature 15–17°C) over the range of citrate concentrations to be used (10 to 100 mM) have been determined experimentally (Szecsody et al. 2009) and simulated with a first-order model (Bailey and Ollis 1986; Bynhildsen and Rosswall 1997). In addition, the microbial biomass has been characterized with depth and position along the Columbia River shoreline, and the relationship between biomass and the citrate biodegradation rate determined (Szecsody et al. 2007). Because Hanford 100-N Area injections typically use river water (~90–95%) along with the concentrated chemicals, microbes in the river water are also injected, which results in a somewhat more uniform citrate biodegradation rate in different aquifer zones.

The specific steps of this remediation technology are as follows:

- injection of Ca-citrate/PO<sub>4</sub> solution
- in situ biodegradation of citrate resulting in apatite [Ca<sub>6</sub>(PO<sub>4</sub>)<sub>10</sub>(OH)<sub>2</sub>] precipitation and coprecipitation of <sup>90</sup>Sr in pore fluid and solids in the treatment zone
- adsorption of <sup>90</sup>Sr by the apatite surface (new <sup>90</sup>Sr migrating into the treated zone from upgradient sources)
- apatite recrystallization with <sup>90</sup>Sr substitution for calcium (permanent)
- radioactive decay of <sup>90</sup>Sr to <sup>90</sup>Y to <sup>90</sup>Zr.

## 2.5 High-Concentration Apatite Amendment Formulation

This section contains 1) a description of the evolution of the Ca-citrate-PO<sub>4</sub> amendment solution from a low- to a high-concentration formulation, 2) specifications for the resulting high-concentration apatite amendment formulation, and 3) a discussion of the quantity of apatite that will be required to meet remedial objectives for <sup>90</sup>Sr sequestration at 100-NR-2.

### 2.5.1 Formulation Development

The original Ca-citrate-PO<sub>4</sub> amendment formulation was based on the stoichiometric ratio of Ca:PO<sub>4</sub> in apatite (5:3), and a Ca:citrate ratio of 1:2.5 to form the aqueous Ca-citrate complex (Moore et al. 2004, 2006). Therefore, the initial Ca-citrate-PO<sub>4</sub> formulation contained the ratios of 4 mM Ca, 10 mM citrate, and 2.4 mM PO<sub>4</sub>. Early laboratory experiments with this formulation (and higher concentrations) did successfully precipitate apatite, but left considerable excess calcium in solution because there is a significant quantity of adsorbed Ca<sup>2+</sup> on sediment minerals (by ion exchange) that exchanges off the surface during solution injection.

Using a Ca-citrate-PO<sub>4</sub> solution to treat <sup>90</sup>Sr requires injecting sufficient phosphate for treatment while avoiding negative side effects. Sufficient mass of phosphate needs to be emplaced in the aquifer to sequester <sup>90</sup>Sr for 300 years, as defined by both mass balance (incorporation of <sup>90</sup>Sr into apatite) and <sup>90</sup>Sr flux rate considerations, as discussed in Section 2.5.2. However, any solution injected into the aquifer that is of higher ionic strength than groundwater will cause some desorption of <sup>90</sup>Sr<sup>2+</sup> (and Sr<sup>2+</sup>, Ca<sup>2+</sup>, Mg<sup>2+</sup>...) from the sediment because 99% of the <sup>90</sup>Sr mass is adsorbed by ion exchange on sediment

minerals. The ion exchange process is a function of the concentration of monovalent and divalent cations in the solution and the total volume injected. The rate of solution injection into the sediment is also important because it influences the movement of solution components. Phosphate exhibits slow sorption and precipitation and phosphate reactions are, therefore, sensitive to injection rate even though cation exchange is relatively rapid and generally invariant with respect to injection rate. To better mimic field conditions, laboratory experiments using 1-m to 10-m-long one-dimensional (1-D) columns at field flow rates, followed by slow groundwater injection were used to approximate field flow conditions and evaluate different Ca-citrate-PO<sub>4</sub> injection formulations.

The primary objectives of 1) injecting sufficient phosphate and 2) minimizing initial <sup>90</sup>Sr mobilization were evaluated in laboratory experiments by testing different injection strategies that included the following:

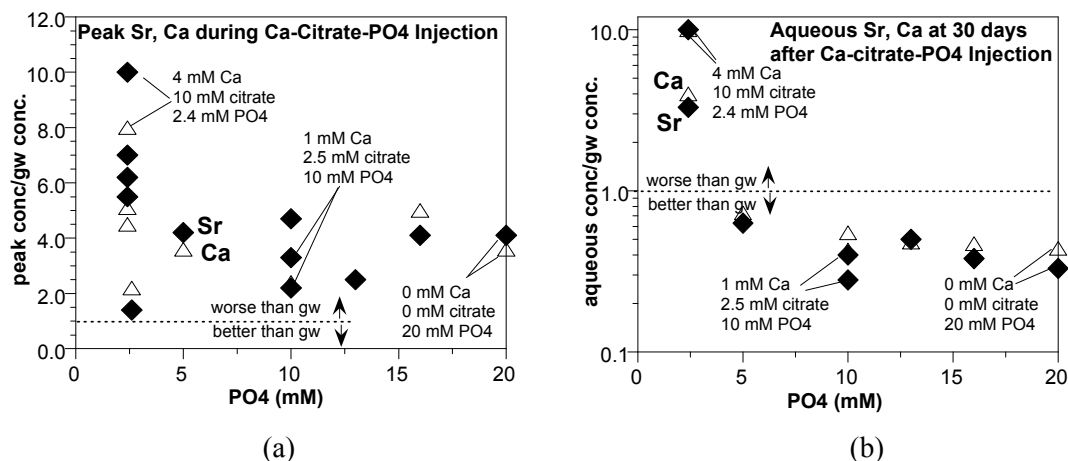
- different Ca-citrate-PO<sub>4</sub> concentrations (original 4:10:2.4 ratio of Ca: citrate: PO<sub>4</sub>)
- injection of phosphate only
- injection of citrate-PO<sub>4</sub> (no Ca)
- sequential injection of a low, then high-concentration Ca-citrate-PO<sub>4</sub> formula
- sequential injection with different formulations
- calcium-poor formulations
- addition of fluoride in to increase the precipitation rate.

Of the injection strategies evaluated, those showing the most promise included 1) sequential injection of low-, then high-concentration Ca-citrate-PO<sub>4</sub> and 2) injection of a calcium-poor formulation of Ca-citrate-PO<sub>4</sub> solution. As discussed in Section 2.3, the original concept for field-scale deployment of the apatite PRB technology was based on sequential low-, then high-concentration treatments. Because the most recent laboratory experiments demonstrated that injection of a high-concentration calcium-poor solution achieved the objectives as well as sequential low-, then high-concentration solution injections (i.e., emplacement of sufficient PO<sub>4</sub> mass, limit short-term <sup>90</sup>Sr mobilization), future field injections will likely use only the high-concentration calcium-poor solution formulation, because it provides for a more cost-effective implementation approach.

Laboratory experiments testing different sequential injection strategies (described in Section 5.8 in the report by Szecsody et al. [2007]) showed that a greater amount of strontium was incorporated into apatite if a calcium-poor formulation was used. In the calcium-carbonate-saturated Hanford groundwater system, there is sufficient calcium adsorbed on the sediment by ion exchange to precipitate apatite for a 35-mM phosphate solution, assuming 100% of the Ca<sup>2+</sup> adsorbed on the sediment would be available for the apatite precipitation. Although this calculation shows that lower calcium mass could be injected with the phosphate, the exact quantity of calcium that needs to be injected depends on the dynamics of the calcium ion exchanging off the minerals during an injection. Injection of any calcium-poor (i.e., less calcium than the 5:3 ratio of Ca:PO<sub>4</sub>) will use some Ca<sup>2+</sup> from the sediment, as well as strontium and <sup>90</sup>Sr, which chemically behave similarly to calcium. Overuse of calcium-poor Ca-citrate-PO<sub>4</sub> solutions would eventually deplete the sediment Ca<sup>2+</sup> content, and the phosphate would have to drift downgradient in the aquifer to a zone with some available calcium for precipitation to occur. The final low-concentration apatite amendment formulation designed to minimize the initial <sup>90</sup>Sr peak was a Ca-citrate-

PO<sub>4</sub> solution with a 1: 2.5:10 ratio (Szecsody et al. 2007), which is 75% depleted in the calcium needed to form apatite (i.e., 75% of the calcium used in apatite formation is desorbed from aquifer sediments).

The decrease in the short-term peak aqueous strontium concentrations, and concentrations at 30 days, was substantial as the formulation was changed from a Ca-citrate-PO<sub>4</sub> ratio of 4:10:2.4 to 1:2.5:10. At the original formulation (2.4 mM PO<sub>4</sub>, ionic strength 96 mM), the peak strontium concentration was 10 times that of the initial groundwater concentration (Figure 2.13a), and the strontium concentration did not decrease by 30 days (Figure 2.13b). With the calcium-poor formulation at 4 times the amount of phosphate (1 mM Ca, 2.5 mM citrate, 10 mM PO<sub>4</sub>, ionic strength = 65 mM), the initial strontium peak

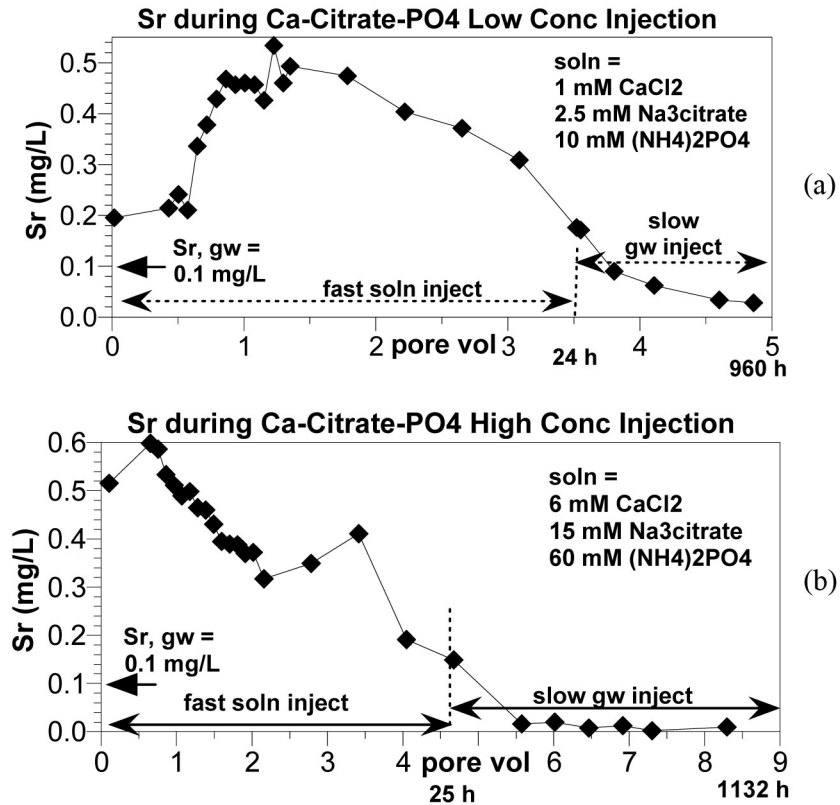


**Figure 2.13.** Sr aqueous peak (a) and 30-day (b) concentrations during Ca-citrate-PO<sub>4</sub> solution injection into 1-D sediment columns. Different ratios of C:citrate:PO<sub>4</sub> were used in these experiments.

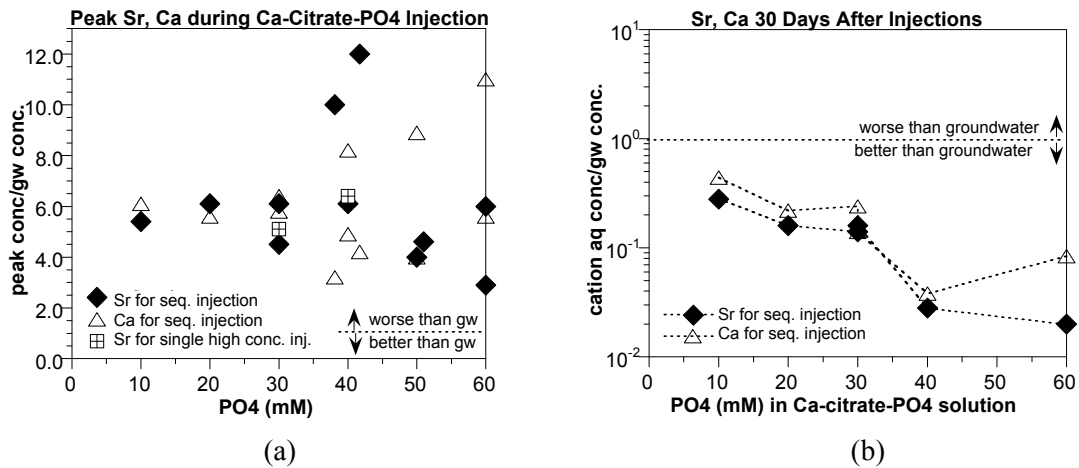
aqueous concentration was 3.3 times that in groundwater, and by 30 days it had decreased to 0.4 times that of groundwater (i.e., due to precipitation using calcium and strontium desorbed from sediment). Phosphate at 20 mM (with no calcium or citrate) injected into the sediment also had similar peak and 30-day aqueous concentrations (Figure 2.13), but was not used as a final low-concentration formulation because with no injected calcium, eventually all of the calcium in the sediment would be depleted, and the injected plume would move down gradient (i.e., the apatite would not precipitate in the zone of interest).

A series of laboratory experiments was conducted to determine an appropriate high-concentration Ca-citrate-PO<sub>4</sub> formulation for use in the PRB-emplacement injections. As shown in Figure 2.14, a low-concentration injection followed by a 1-year wait, then high-concentration (60 mM PO<sub>4</sub>) resulted in decreased <sup>90</sup>Sr aqueous peak concentrations. In the initial low-concentration experiment (Figure 2.14a), the strontium peak aqueous concentration (0.5 mg/L) was 5 times that of the initial groundwater concentration (0.1 mg/L) and by 30 days had decreased to 0.28 times the initial concentration. In the second sequential injection (Figure 2.14b), the initial strontium peak aqueous concentration was 6 times that of groundwater (0.6 mg/L) and by 30 days the aqueous concentration was 0.02 times that of groundwater.

A concentration range of 20 mM to 60 mM phosphate in Ca-citrate-PO<sub>4</sub> solutions (with the relative ratios of Ca:citrate:PO<sub>4</sub> of 1:2.5:10) was evaluated in laboratory experiments using sediments that were previously treated with a 10-mM phosphate solution (shown in Figure 2.14a) and allowed to incorporate



**Figure 2.14.** Sr aqueous concentration in 1-D column during sequential injection of (a) low Ca-citrate-PO<sub>4</sub> solution, followed by a 1-year wait, then (b) high-concentration solution injection



**Figure 2.15.** Sr and calcium aqueous peak (a) and 30-day (b) concentrations during Ca-citrate-PO<sub>4</sub> sequential solution injection into 1-D sediment

strontium for 1 year. As the phosphate concentration was increased from 10 mM to 60 mM, the peak aqueous strontium concentration (Figure 2.15a, large black diamonds) increased (i.e., strontium peak 6x to 12x). In the same experiments the calcium peak aqueous concentrations also increased (open triangles). At phosphate concentrations greater than 30 mM, apatite precipitation was more rapid (within hours), so precipitation was occurring in some effluent tubes (accounting for the spread in peak strontium and calcium values, Figure 2.15a). This precipitation artifact was eliminated by preacidifying the tubes to prevent precipitation (i.e., apatite dissolves under highly acidic conditions). In comparison to sequential low- than high-concentration injections, a single high-concentration Ca-citrate-PO<sub>4</sub> injection (squares, Figure 2.15a) shows similar strontium peak concentrations (data only at 30 and 40 mM PO<sub>4</sub>).

After 30 days, substantially lower strontium and calcium aqueous concentrations were observed for the higher Ca-citrate-PO<sub>4</sub> concentrations in the second injection (Figure 2.14b). Higher phosphate injections precipitated more rapidly, and because the solution was calcium-poor, both calcium and strontium were removed more quickly from solution.

Alternative injection strategies in addition to the sequential low-, then high-concentration Ca-citrate-PO<sub>4</sub> formulation were also evaluated in laboratory experiments. Injection of 20-mM PO<sub>4</sub> only (shown in Figure 2.13) produced similar peak and 30-day strontium concentrations as the calcium-poor Ca-citrate-PO<sub>4</sub> solution. As described earlier, two disadvantages of injecting only phosphate include 1) depletion of sediment Ca<sup>2+</sup>, which would limit the total phosphate mass that could be injected, and 2) overly rapid precipitation rates that limit the lateral extent of phosphate transport. The Ca-citrate complex maintains some injected calcium as a solution complex until the citrate is biodegraded, thus limiting the apatite precipitation rate (i.e., allowing time for the solution to be injected before precipitation occurs). Injection of a single high-concentration Ca-citrate-PO<sub>4</sub> solution (squares, Figure 2.15a) produced similar strontium peak concentration as sequential low-, then high-concentration injections, and is more cost effective at the field scale. In another series of experiments, F<sup>-</sup> was added to the Ca-citrate-PO<sub>4</sub> injection solution at 0.21-mM (drinking water limit) and at 2.1-mM concentrations. The F<sup>-</sup> addition had little effect on the initial strontium and calcium peak concentration, but increased the apatite precipitation rate, so at 30 days after the injection, both strontium and calcium concentrations were 10x or more lower than a corresponding injection experiment without F<sup>-</sup> addition.

The final high-concentration formulation (3.6 mM Ca, 9 mM citrate, 40 mM PO<sub>4</sub>) is a calcium-poor solution of sufficient phosphate mass such that two high-concentration injections will provide sufficient apatite mass for 300 years of <sup>90</sup>Sr sequestration. The solubility limit of a single high-concentration injection is ~70 mM PO<sub>4</sub> in a Ca-citrate-PO<sub>4</sub> solution). The initial aqueous <sup>90</sup>Sr peak concentration for this high-concentration injection is predicted at 6 to 12 times the initial groundwater concentration (i.e., Figure 2.15a), and once groundwater has flushed into the injection zone (i.e., approximated in Figure 2.15b), the <sup>90</sup>Sr peak is predicted to be <1/10 the pre-injection <sup>90</sup>Sr concentration. Based on laboratory experiments, a high-concentration formulation between 40 and 60 mM PO<sub>4</sub> would produce similar initial <sup>90</sup>Sr peak and post-peak aqueous concentrations (Figure 2.15), but the higher precipitation rate of the 60-mM PO<sub>4</sub> solution would increase the likelihood of precipitates forming in the injection systems and/or well screen, and possibly limiting the lateral extent of treatment.



## 2.5.2 High-Concentration Formulation

Based on the bench- and field-scale treatability tests conducted to date using both low- and high-concentration apatite amendment formulations, the most favorable formulation identified for field-scale deployment of the technology consists of 3.6-mM calcium, 9-mM citrate, and 40-mM phosphate. This amendment solution was identified as the best formulation for meeting the following objectives:

1) minimize the number of injection operations required, 2) minimize short-term increases in  $^{90}\text{Sr}$  concentrations associated with injection of high-ionic-strength solutions, and 3) keep amendment formulations well below solubility limits to reduce potential for operational challenges associated with solution stability. The recipe for the high-concentration apatite injection solution is as follows:

1. 9.0 mM trisodium citrate [ $\text{HOC}(\text{COONa})(\text{CH}_2\text{COONa})_2 \cdot 2\text{H}_2\text{O}$ ] formula weight (FW) 294.1 g/mol
  - also called sodium citrate dihydrate, American Chemical Society (ACS) registry **6132-04-3**
  - granular is more soluble than powdered
  - reagent-grade (quality) or equivalent for the citrate: USP/FCC (United States Pharmacopeia pentahydrate) (lower grades contain up to 5 ppm heavy metals)
2. 3.6 mM calcium chloride ( $\text{CaCl}_2$ ), FW 110.98 g/mol
  - Reagent-grade (quality) or equivalent: certified ACS, ACS registry **10043-52-4** (lower grades can contain 20 ppm lead)
3. 32.4 mM disodium hydrogenphosphate ( $\text{Na}_2\text{HPO}_4$ ), FW 141.96 g/mol
  - also called disodium phosphate, anhydrous
  - reagent-grade (quality) or equivalent: certified ACS, ACS registry **7558-79-4** (lower grades can contain extra NaOH, which is only a small problem, changes pH and ionic strength)
4. 5.6 mM sodium dihydrogenphosphate ( $\text{NaH}_2\text{PO}_4$ ), FW 119.98 g/mol
  - also called monosodium phosphate, anhydrous
  - reagent-grade or equivalent: certified ACS grade, ACS registry **7558-80-7** (lower grades can contain 8 ppm arsenic and 10 ppm heavy metals)
5. 2.0 mM diammonium hydrogenphosphate [ $(\text{NH}_4)_2\text{HPO}_4$ ], FW 132.1 g/mol
  - also called diammonium phosphate
  - granular is more soluble than powdered
  - reagent-grade (quality) or equivalent: certified ACS, ACS registry **7783-28-0**
6. 1.0 mM sodium bromide ( $\text{NaBr}$ ), FW 102.90 g/mol
  - Reagent-grade (quality) or equivalent: certified ACS, ACS registry **7647-15-6**.

## 2.5.3 Mass of Apatite Needed for Hanford 100-N Area

Two factors control the amount of apatite needed to sequester Sr in the Hanford 100-N Area. First, from the standpoint of mass balance, a specific amount of apatite is needed to remove all strontium and Sr

from groundwater over the next 300 years (i.e., 10 half-lives of Sr decay, half-life 29.1 years). This calculation is dependent on the crystal substitution of strontium for calcium in apatite. If a 10% substitution is assumed, then 1.7 mg of apatite is sufficient to sequester strontium and Sr from the estimated 3300 pore volumes of water that will flow through an apatite-laden zone. This calculation assumes an average groundwater flow rate of 0.3 m/day (1 ft/day) and a 10-m (32-ft) apatite PRB thickness. Electron microprobe analysis of the strontium substitution in apatite precipitate (Szecsody et al. 2009) showed that after 1.3 years there was a 16.1% substitution of strontium for calcium in microcrystalline (20-40 micron particles composed of <1 micron crystals) apatite, and a 9.1% substitution of strontium for calcium in crystalline apatite (20-40 micron, single crystal). The majority of the apatite precipitating in sediment was microcrystalline, so the assumption of a 10% substitution is achievable. The 1.7-mg apatite/g of sediment does occupy some pore space in the aquifer, which has an average field porosity of 20%. Given crystal lattice dimensions of 9.3 Å by 6.89 Å (assume a cylinder of dimensions  $7.5 \times 10^{-21} \text{ cm}^3/\text{atom}$ ), the 1.7 mg apatite/g sediment would occupy 13.6% of the pore space, so some degree of permeability decrease would be expected.

The second factor that controls the amount of apatite needed to sequester Sr is the rate of incorporation. This permeable reactive barrier concept, which relies on emplacement of apatite solids in the aquifer, is viable only if the natural groundwater flux rate of strontium and Sr ( $1.36 \times 10^{-6} \text{ mmol strontium/day/cm}^2$ ) is slower than the removal rate of strontium and Sr by apatite. If the groundwater flow rate is too high, even highly sorbing strontium and Sr could advect through the apatite-laden zone more quickly than it is removed. The way to circumvent this issue is to have additional apatite in the groundwater system (i.e., greater than the amount needed based on the mass balance calculation above) to essentially remove Sr at an increased rate. To assess possible implementation limitations associated with the rate of Sr incorporation, numerous experiments were conducted to clearly define the rates at which strontium and Sr are incorporated into the crystal structure of apatite. Although the strontium sorption to apatite is very high ( $K_d = 1370 \text{ cm}^3/\text{g}$  or 55 times greater than to sediment), because the total mass of apatite is small in the system, sorption to apatite does not remove significant strontium or Sr mass. However, strontium incorporation into apatite does remove Sr from aqueous solution and ion exchange sites, so it is no longer labile in the subsurface sediments. For the initial low-concentration treatments (i.e., 10 mM  $\text{PO}_4$  injected resulting in 0.38 mg apatite/g sediment), the strontium uptake rate was  $8.8 \times 10^{-6} \text{ mmol Sr/day/cm}^2$ , or 6.5 times the average strontium groundwater flux rate ( $1.36 \times 10^{-6} \text{ mmol Sr/day/cm}^2$ ). Therefore, on a rate basis, all of the strontium (and Sr) would be consumed by the apatite in the barrier. However, for zones of higher groundwater flux (e.g., 10 to 100 times), the groundwater strontium flux rate would be expected to exceed the barrier uptake rate for this low apatite loading. In addition, this low apatite loading would not result in sufficient treatment capacity for a 300-year barrier longevity. At an apatite content of 1.7 mg apatite/g of sediment, the strontium uptake rate was  $8.8 \times 10^{-5} \text{ mmol Sr/day/cm}^2$  or 65 times the average strontium groundwater flux rate ( $1.36 \times 10^{-6} \text{ mmol } ^{90}\text{Sr/day/cm}^2$ ). At this apatite content (needed from a mass balance perspective), Sr would be incorporated into apatite more rapidly than the average groundwater flow rate and most high-flow events.

This target apatite content (1.7 mg apatite/g of sediment) corresponds to a pore volume amendment concentration, on a molar basis, of 90 mM of phosphate precipitated in sediment with no retardation. As indicated above, because of phosphate solubility limits and other technical considerations, the final high-concentration formulation was specified at 40 mM phosphate. Phosphate retardation associated with adsorption and precipitation reactions occurs during amendment injection ( $R_f \sim 2$ , range 1.6 to 2.4). Assuming 1) injection of 40 mM phosphate (and previous 10 mM low-concentration treatment),

2) average phosphate retardation, and 3) an even spatial distribution of phosphate in the Hanford and Ringold formations, the resulting apatite loading would be 1.9 mg apatite/g of sediment. However, because higher apatite loading may be needed in this hydrodynamically complex setting to provide sufficient Sr incorporation rates, more than one injection may be required.

An evaluation based on sediment core samples that are planned for collection in November 2009, approximately 1 year after the high-concentration treatments, will be used to quantify the amount of apatite formation resulting from the sequential low- followed by high-concentration treatments performed to date (i.e., an initial 1.0 mM Ca, 2.5 mM citrate, and 10 mM PO<sub>4</sub> formulation followed by 3.6 mM Ca, 9 mM citrate, and 40 mM PO<sub>4</sub>). Based on these results, a determination will be made regarding the total volume of amendment solution, and thus the number of treatments, that will be required for full-scale deployment of the technology. If multiple high-concentration treatments are required, the timing of these injections must be considered. Because the high-concentration injection formulation uses a considerable fraction of the adsorbed Ca<sup>2+</sup> from the sediment (roughly half of the adsorbed Ca<sup>2+</sup>), sufficient time is needed for the system to reach equilibrium with the reintroduction of calcium from upgradient. Given a retardation factor for calcium of roughly the same order of magnitude as strontium (~100 to 120) and approximately 10 pore volumes per year passing through the apatite PRB, an estimated 5 years is needed for calcium to reach equilibrium in the treatment zone.

Development of the final injection approach for full-scale deployment of the PRB technology will consider the following design criteria: 1) emplace sufficient apatite to meet long-term remedial objectives, 2) limit permeability reduction by emplacing only the apatite content needed to meet remedial objectives, 3) limit short-term increases in Sr concentration associated with the injection of high-ionic-strength solutions, and 4) implementation cost.



## 3.0 PRB-Emplacement Operations

As discussed in Section 2.3, the field-testing approach adopted for implementation of the apatite PRB technology involved initial pilot-scale field testing that was used to refine the injection design for subsequent barrier well treatments. This same approach was adopted during initial testing and subsequent PRB-emplacement injections that used the high-concentration apatite amendment formulation. This section contains a description of the injection design, operations, and operational performance for the high-concentration pilot-scale testing and PRB-emplacement injection operations.

### 3.1 Injection Design

Based on design analysis and chemical arrival responses observed during low-concentration pilot-scale testing and PRB treatment operations (Williams et al. 2008) and pilot-scale field tests with the high-concentration solution, an injection volume of 120,000 gallons of apatite amendment solution was specified for each well (or Hanford/Ringold well pair). This specified volume was demonstrated to provide sufficient radial extent of treatment at the targeted radial extent of 20 ft, resulting in effective overlap coverage between adjacent injection wells. Over the upstream most one third of the barrier (199-N-138 and 199-N-141, Figure 2.3), a single 120,000-gallon injection was conducted to treat the Hanford and Ringold formations simultaneously. Over the downstream portion of the barrier, two separate injection operations of approximately 60,000 gallons each were required for targeted treatment of the Hanford and Ringold formations. This portion of the barrier was characterized by generally higher well-specific capacity and a larger hydraulic conductivity contrast between the Hanford and Ringold formations (with the Hanford formation hydraulic conductivity values higher relative to the upstream values). The implication of these conditions is that injections over the downstream portion of the barrier must be performed as two separate operations, one targeting the Ringold Formation, the other targeting the Hanford formation; injections were split between fully screened wells (which focus treatment predominantly on the Hanford formation) and wells screened across only the Ringold Formation.

An injection rate of 40 gpm was specified for treatment of the upstream portion of the barrier based on the hydraulic performance of injection wells observed during previous barrier treatment operations and injection design analyses indicating that these rates were sufficient for effective delivery of the phosphate amendment. During treatment of the downstream portion of the barrier, an injection rate of 20 gpm was specified for both the Hanford-inclusive and Ringold-only injection wells (for a total of 40 gpm). At this rate, treatment at each well location took ~50 hours to complete.

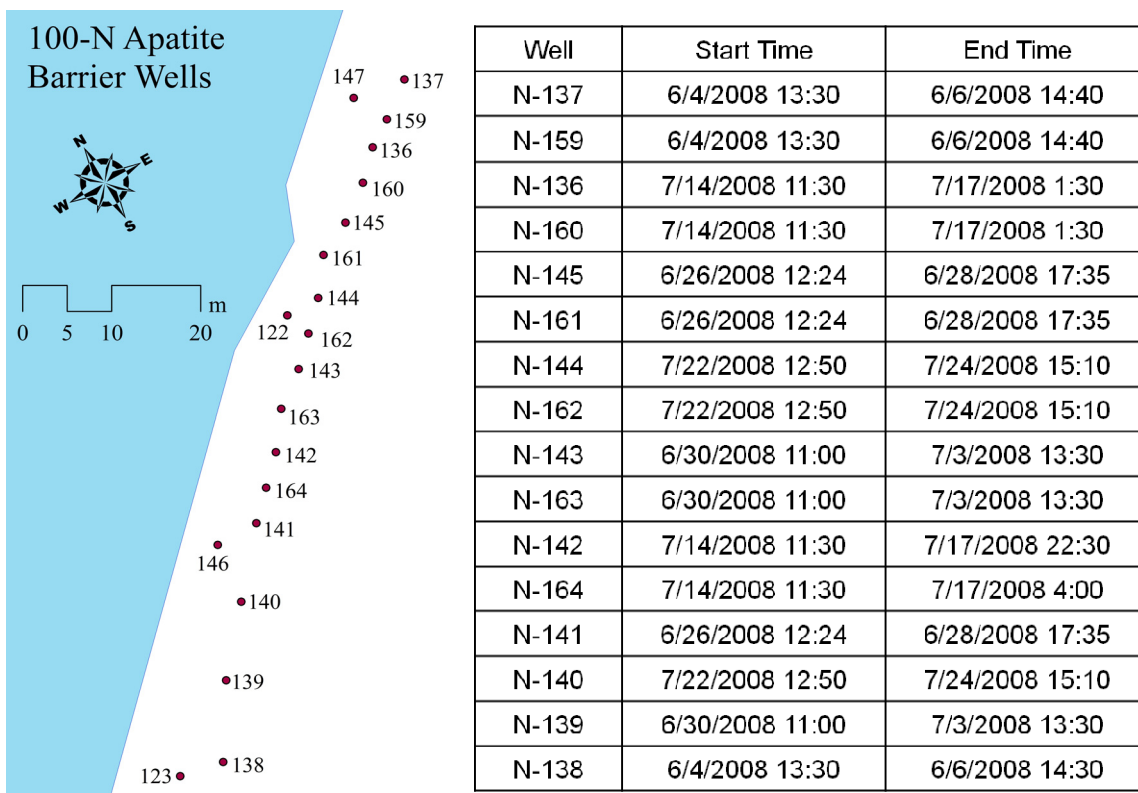
Both the upstream portion of the barrier and the Hanford interval over the downstream portion of the barrier requires treatment during high-Columbia-River-stage conditions to treat as high in the Hanford formation profile as possible. It was originally thought that treatment of the Ringold-only injection wells could be conducted during any river-stage condition; however as discussed below, amendment arrival response data collected during the high-concentration injections indicate that treatment of the Ringold Formation is most effective when conducted in Ringold-only injection wells at low-Columbia-River stage. A detailed description of the injection and monitoring equipment, and the aqueous sampling and analysis requirements for these barrier-emplacement operations is provided by Williams et al. (2008). A description of the high-concentration amendment formulation is provided in Section 1.5.2.

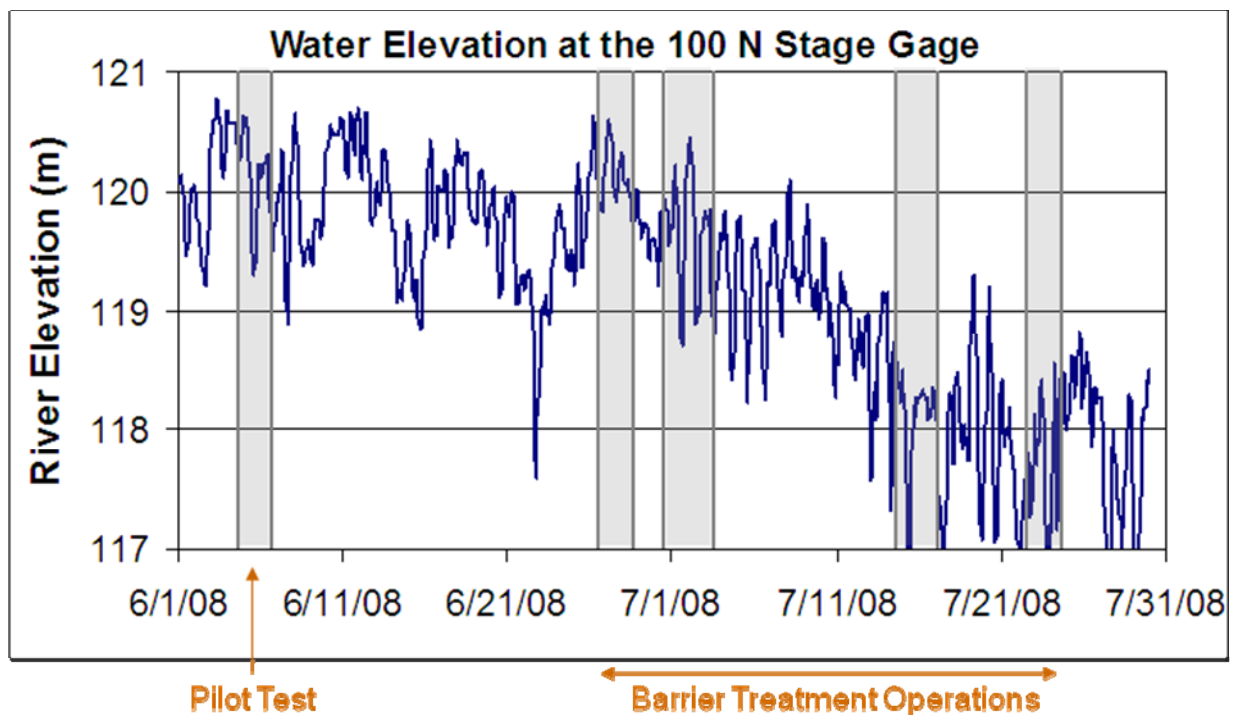
## 3.2 Description of Injection Operations

The section describes the high-concentration apatite solution injections conducted during the initial phase of barrier-emplacement operations (i.e., post-low-concentration pretreatment) for the 91-m (300-ft)-long apatite PRB section. Injection operations were conducted in 16 wells in June and July 2008 (Table 3.1), the original 10 injection wells completed over the Hanford formation and upper contaminated portion of the Ringold Formation, and 6 additional Ringold-only injection wells.

Injections were conducted during both low- and high-river-stage conditions (see Figure 3.1 and Table 3.2). River elevation typically varied by as much as 1 m during the injection and reaction period. The targeted water-level elevation for full treatment of the Hanford formation sediments (i.e., the approximate contact elevation between the undisturbed Hanford formation sediments and the road fill materials) is approximately 120 m. As shown in Figure 3.1, the last two barrier-emplacement campaigns in mid to late July were conducted during river-stage conditions that were up to 2 m below this design target, when only a small portion of the Hanford formation sediments were saturated. Of the 10 Hanford-inclusive injections wells, four were treated when the average water-level elevation was approximately 120 m (full Hanford treatment), two were treated to an elevation of approximately 119 m, and four were treated to an elevation of approximately 118 m (~2 m below the targeted water-level elevation).

**Table 3.1.** Injection start and stop times for initial high-concentration barrier-emplacement operations





**Figure 3.1.** Columbia-River-stage conditions during barrier-emplacement operations

**Table 3.2.** Average river stage during injection operations and one-week reaction period at each well location (Ringold-only injection wells highlighted in gray)

Well	Injection Period			Injection + 7 day reaction period		
	Average	Maximum	Minimum	Average	Maximum	Minimum
N-137	120.1	120.6	119.3	120.1	120.7	118.9
N-159	120.1	120.6	119.3	120.1	120.7	118.9
N-136	118.1	118.8	116.6	117.9	119.3	116.6
N-160	118.1	118.8	116.6	117.9	119.3	116.6
N-145	120.1	120.6	119.7	119.7	120.6	118.2
N-161	120.1	120.6	119.7	119.7	120.6	118.2
N-144	117.7	118.6	116.9	117.8	118.8	116.2
N-162	117.7	118.6	116.9	117.8	118.8	116.2
N-143	119.7	120.4	118.7	119.4	120.4	118.2
N-163	119.7	120.4	118.7	119.4	120.4	118.2
N-142	118.0	118.8	116.6	117.9	119.3	116.6
N-164	118.1	118.8	116.6	117.9	119.3	116.6
N-141	120.1	120.6	119.7	119.7	120.6	118.2
N-140	117.7	118.6	116.9	117.8	118.8	116.2
N-139	119.7	120.4	118.7	119.4	120.4	118.2
N-138	120.1	120.6	119.3	120.1	120.7	118.9

The injection volume, amendment mass, and the average amendment concentration for each injection, were determined by monitoring concentrations within the injection well, monitoring flow rates of the injection stream and concentrated feed solutions, and measuring the undiluted chemical concentrations and volumes. A summary of the operational data for all 16 injections is provided in Table 3.3 with operational data presented as a percentage of the design specification shown in Table 3.4. These barrier-emplacement operations were characterized by generally higher amendment concentrations and thus lower total solution volumes than specified in the injection design. However, solution volumes, concentrations, and calculated amendment mass values in most cases were within acceptable operational tolerance limits. The one notable exception was at Ringold-only well 199-N-164, where precipitate formation in the phosphate tanker solution resulted in well screen plugging, pressure buildup, and ultimately early termination of the injection.

Several additional injection wells showed evidence for varying degrees of well screen or near-well filter pack/formational plugging during treatment (Table 3.5). Excessive buildup during these injections was mitigated by decreasing the injection rate and extending the injection duration. Although this approach allowed the injections to continue until the total solution volume specification was met, the lower injection rates result in increased phosphate retardation and thus, decreased radial extent of apatite formation. The observed plugging is thought to be associated with tanker solution stability problems that resulted in precipitate formation in the concentrated phosphate tanker solution. Evidence for injection-related plugging of a sufficient extent to significantly affect the ability to sustain target injection rates was observed at four locations (N-139, N-142, N-143, and N-161). Three additional Ringold-only injection wells (N-160, N-163, and N-164) exhibited low specific capacity prior to treatment, so the impacts of treatment-related plugging cannot be effectively assessed. Regardless of the cause, these seven well

**Table 3.3.** Operational data summary for high-concentration injections at 16 injection well locations

Well Name	Total Solution Injection Volume (gallons)	Average Injection Concentration (mg/L)			Total Chemical Injection Mass (kg)		
		Calcium	Citrate	Phosphate	Calcium	Citrate	Phosphate
N-137 (H)	57,559	125	1740	3732	27.2	379	812
N-159 (R)	57,559	125	1740	3732	27.2	379	812
N-136 (H)	58,829	182	1775	3938	40.4	395	876
N-160 (R)	58,829	182	1775	3938	40.4	395	876
N-145 (H)	57,328	166	1663	3907	36.0	360	847
N-161 (R)	57,328	166	1663	3907	36.0	360	847
N-144 (H)	59,962	170	1374	3968	38.6	312	899
N-162 (R)	59,962	170	1374	3968	38.6	312	899
N-143 (H)	54,545	156	1973	4385	32.2	407	904
N-163 (R)	54,545	156	1973	4385	32.2	407	904
N-142 (H)	60,608	179	1710	4128	41.1	392	946
N-164 (R)	35,035	179	1710	4128	23.7	226	547
<b>Design Specification</b>	<b>60,000</b>	<b>140</b>	<b>1654</b>	<b>3323</b>	<b>31.8</b>	<b>375</b>	<b>754</b>
N-141	113,114	167	1702	3906	71.3	728	1670
N-140	115,650	170	1685	3824	74.1	736	1672
N-139	105,470	213	1943	4425	84.8	775	1764
N-138	116,352	124	1686	4011	54.4	741	1764
<b>Design Specification</b>	<b>120,000</b>	<b>140</b>	<b>1654</b>	<b>3323</b>	<b>63.5</b>	<b>750</b>	<b>1507</b>



**Table 3.4.** Summary of operational data presented as a percentage of the design specification

Well Name	Total Solution Injection Volume (gallons)	Average Injection Concentration (mg/L)			Total Chemical Injection Mass (kg)		
		Calcium	Citrate	Phosphate	Calcium	Citrate	Phosphate
N-137 (H)	96%	89%	105%	112%	86%	101%	108%
N-159 (R)	96%	89%	105%	112%	86%	101%	108%
N-136 (H)	98%	130%	107%	119%	127%	105%	116%
N-160 (R)	98%	130%	107%	119%	127%	105%	116%
N-145 (H)	96%	119%	101%	118%	114%	96%	112%
N-161 (R)	96%	119%	101%	118%	114%	96%	112%
N-144 (H)	100%	122%	83%	119%	122%	83%	119%
N-162 (R)	100%	122%	83%	119%	122%	83%	119%
N-143 (H)	91%	112%	119%	132%	102%	108%	120%
N-163 (R)	91%	112%	119%	132%	102%	108%	120%
N-142 (H)	101%	128%	103%	124%	129%	104%	125%
N-164 (R)	58%	128%	103%	124%	75%	60%	73%
N-141	94%	119%	103%	118%	112%	97%	111%
N-140	96%	121%	102%	115%	117%	98%	111%
N-139	88%	152%	117%	133%	134%	103%	117%
N-138	97%	88%	102%	121%	86%	99%	117%
<b>Average</b>	<b>96%</b>	<b>114%</b>	<b>103%</b>	<b>120%</b>	<b>110%</b>	<b>99%</b>	<b>115%</b>
<b>Standard Deviation</b>	<b>3%</b>	<b>15%</b>	<b>12%</b>	<b>7%</b>	<b>16%</b>	<b>9%</b>	<b>5%</b>

**Table 3.5.** Pressure buildup observations and associated injection rate limitations encountered during barrier-emplacement operations (Ringold-only injection wells highlighted in gray)

Injection Well	Injection Duration (hr)	Pressure Buildup After 2 Hours (ft)	Pressure Bulidup at End of Injection (ft)	Start Flow Rate (gpm)	End Flow Rate (gpm)
N-137	49.2	1.6	0.3	20	20
N-159	49.2	27.2	14.4	20	20
N-136	62.0	4.9	23	20	10
N-160	62.0	39.3	>47 <sup>(a)</sup>	20	10
N-145	53.2	4.6	4.9	20	15
N-161	53.2	10.5	>42 <sup>(a)</sup>	20	15
N-144	50.3	1.6	17.4	20	20
N-162	50.3	11.5	22.6	20	20
N-143	74.5	1	30.5	20	4.45
N-163	74.5	>42 <sup>(a)</sup>	>42 <sup>(a)</sup>	20	4.45
N-142	64.5	6.6	>41 <sup>(a)</sup>	20	4.05
N-164	83.0	>41 <sup>(a)</sup>	>41 <sup>(a)</sup>	20	4.05
N-141	53.2	5.2	4.6	40	30
N-140	50.3	8.5	15.1	40	40
N-139	74.5	9.8	53.5	40	7.3
N-138	49.0	9.2	9.2	40	40
(a) Sensor over range					

locations should be considered for aggressive well maintenance and development procedures, including the potential for using acidic solutions to dissolve precipitates formed on the well screens, prior to any future barrier-emplacement operations.

### 3.3 Operational Performance

Design specifications for the PRB-emplacement injections stipulated that the chemical concentrations should be at least 50% of injection concentration at 6.1 m (20 ft) from each injection well (Williams et al. 2008). This is considered a sufficient radial extent of treatment to provide overlap of treatment between adjacent injection wells. However, with the exception of the two pilot test sites at either end of the barrier, no overlap zone monitoring is available. To address this limitation, arrival data from adjacent injection wells (9.1-m [30-ft spacing]) were used as an indicator. To account for the increase in radial distance to this monitoring point, the phosphate concentration metric for arrival at adjacent injection wells was reduced to 20% to 30% of the injection concentration (from 50% at a 6.1-m [20-ft] distance).

Specific conductance (SpC) and phosphate concentration arrival values at adjacent monitoring locations are presented in Table 3.6. In addition to this arrival summary, full SpC arrival responses at available adjacent monitoring locations during each of these injections are provided in Appendix A. As expected, phosphate transport was somewhat retarded relative to the bulk solution (as indicated by comparison of the SpC and phosphate concentration measurements presented in Table 3.6). The SpC in adjacent wells was consistently closer to injection well values than the phosphate concentration. Thus, the phosphate concentration was considered a better indicator of treatment efficiency than SpC. Evaluation of treatment distribution effectiveness was hampered by the timing of the injections, with elevated amendment concentrations from previous injections in adjacent wells limiting the ability to assess arrival response. An inclusive evaluation of arrival responses at all injection well locations indicates generally satisfactory treatment. However, arrivals in Ringold Formation were less radially extensive than those observed for the Hanford formation. Ringold treatment performance may be improved by performing injections during low-Columbia-River-stage conditions. This approach is supported by the observation that the two highest percent arrivals in Ringold Formation monitoring locations (N-160 and N-162) occurred during the lowest river-stage conditions.

Operational performance measures that should be considered when identifying candidate wells for supplemental treatment (i.e., reinjection to address treatment deficiencies) include 1) amendment volume and mass injected, 2) amendment arrival at adjacent wells, 3) water-level elevation during treatment, and 4) injection rate limitations associated with well plugging. An operational performance matrix is shown in Table 3.7, which provides a summary of operational effectiveness at each injection well location. Injection design criteria were not fully met at 8 of the 16 injection well locations (well locations highlighted in green), with the primary deficiency at 4 of 8 locations being the limited vertical extent of Hanford formation treatment due to low river-stage conditions during the injection. Wells whose extent of treatment did not meet design criteria should be considered for retreatment, or at a minimum, be placed on a watch list to identify premature  $^{90}\text{Sr}$  breakthrough in a timely manner.

Although injection design criteria were not fully met at a significant number of well locations, aqueous performance assessment monitoring data collected to date (see Section 3.0) indicate good barrier performance, providing evidence that the apatite PRB technology may be relatively robust and able to perform effectively under the geohydrologic and geochemical heterogeneities present at field scale. A

more definitive assessment of barrier performance will be based on the analysis of post-treatment sediment core samples to quantify apatite content and distribution. Collection of core samples is currently planned for November 2009, approximately 1 year after completion of high-concentration treatments.

**Table 3.6.** Final specific conductance measurements and amendment arrival concentrations at adjacent monitoring locations (9.1-m [30-ft] radial distance unless otherwise noted) as a percentage of the injection solution concentration

Injection Well	Upstream Well		Downstream Well	
	Percent of Injection Stream-Specific Conductance	Percent of Injection Stream PO <sub>4</sub> Concentration	Percent of Injection Stream-Specific Conductance	Percent of Injection Stream PO <sub>4</sub> Concentration
N-137 <sup>(a)</sup>	100	100	65 @ 15 ft	60 @ 15 ft
N-159 <sup>(b)</sup>	<sup>(c)</sup>	<sup>(c)</sup>	30	20
N-136 <sup>(a)</sup>	85 <sup>(d)</sup>	70 <sup>(d)(f)</sup>	<sup>(d)</sup>	<60 <sup>(d)(e)(f)</sup>
N-160 <sup>(b)</sup>	<sup>(d)</sup>	<25 <sup>(d)(e)(f)</sup>	90	35 <sup>(f)</sup>
N-145 <sup>(a)</sup>	95	95	95	90
N-161 <sup>(b)</sup>	0	0	15	7
N-144 <sup>(a)</sup>	<sup>(d)</sup>	<40 <sup>(d)(e)</sup>	<sup>(d)</sup>	<25 <sup>(d)(e)</sup>
N-162 <sup>(b)</sup>	<sup>(d)</sup>	<30 <sup>(d)(e)</sup>	90	80
N-143 <sup>(a)</sup>	85	70 <sup>(f)</sup>	80 <sup>(d)</sup>	65 <sup>(f)</sup>
N-163 <sup>(b)</sup>	5	1 <sup>(f)</sup>	10	1 <sup>(f)</sup>
N-142 <sup>(a)</sup>	85 <sup>(d)</sup>	60 <sup>(d)(f)</sup>	<sup>(d)</sup>	<50 <sup>(d)(e)(f)</sup>
N-164 <sup>(b)</sup>	<sup>(c)</sup>	<sup>(c)</sup>	<sup>(d)</sup>	<30 <sup>(d)(e)(f)</sup>
N-141 <sup>(a)</sup>	30	5	30	20%
N-141 <sup>(b)</sup>	<sup>(c)</sup>	<sup>(c)</sup>	10 @ 15 ft	5 @ 15 ft
N-140	<sup>(d)</sup>	<sup>(d)</sup>	<sup>(d)</sup>	<40 <sup>(d)(e)</sup>
N-139 <sup>(a)</sup>	70 (90 @ 15 ft)	50 (60 @ 15 ft) <sup>(f)</sup>	70	50 <sup>(f)</sup>
N-139 <sup>(b)</sup>	70 @ 15 ft	40 @ 15 ft <sup>(f)</sup>	<sup>(c)</sup>	<sup>(c)</sup>
N-138 <sup>(a)</sup>	<sup>(c)</sup>	<sup>(c)</sup>	100	100
N-138 <sup>(b)</sup>	<sup>(c)</sup>	<sup>(c)</sup>	45 @ 15 ft	40 @ 15 ft
(a) Hanford formation treatment (b) Ringold Formation treatment (c) No wells available (d) Previous injections interfered with monitoring results (e) No conclusive evidence for arrival; reported maximum concentration may be associated with previous injections (f) PO <sub>4</sub> arrival may have been impacted by reduced injection rate due to well plugging				

**Table 3.7.** Operational performance summary assessing compliance with injection design criteria (well locations where design criteria were not fully met are highlighted in green)

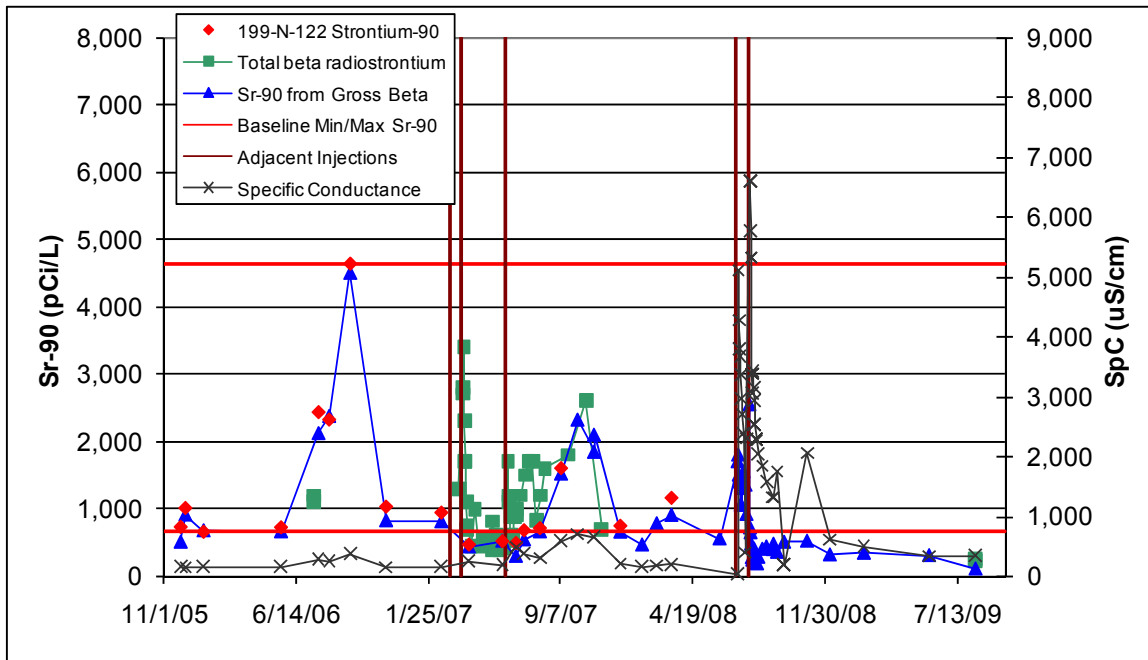
Injection Well	Compliance with Injection Design Criteria				
	Injection Volume	Injection Mass	Radial Extent of Treatment - Hanford	Radial Extent of Treatment - Ringold	Vertical Extent of Hanford Treated <sup>(a)</sup>
N-137	Yes	Yes	Yes	--	103%
N-159	Yes	Yes	--	Yes	--
N-136	Yes	Yes	Yes	--	16%
N-160	Yes	Yes	--	Yes	--
N-145	Yes	Yes	Yes	--	86%
N-161	Yes	Yes	--	No	--
N-144	Yes	Yes	Inconclusive	--	14%
N-162	Yes	Yes	--	Yes	--
N-143	Yes	Yes	Yes	--	75%
N-163	Yes	Yes	--	No	--
N-142	Yes	Yes	Inconclusive	--	16%
N-164	No	No	--	Inconclusive	--
N-141	Yes	Yes	No	No	86%
N-140	Yes	Yes	Inconclusive	Inconclusive	14%
N-139	Yes	Yes	Yes	Yes	75%
N-138	Yes	Yes	Yes	Yes	103%
(a) Assumes Hanford formation extends between 117.5 and 120 m elevation. Average river stage over injection period and 7-day reaction period used.					

## 4.0 Performance Assessment Monitoring

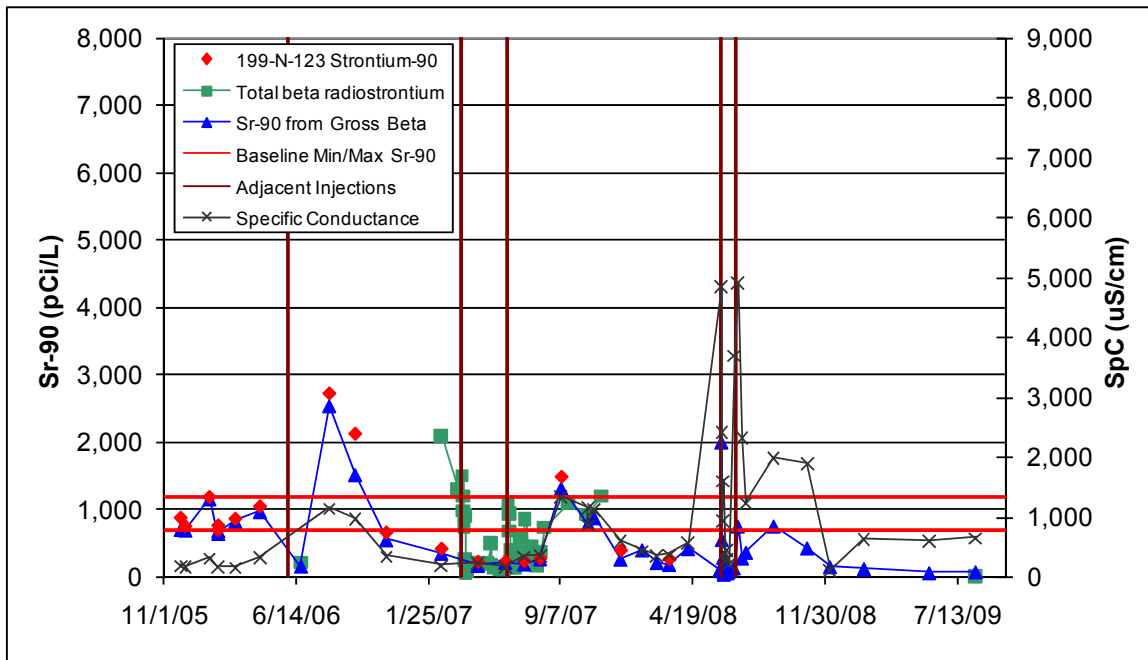
To date, performance assessment of the high-concentration treatments is based on approximately 1 year of groundwater monitoring data. Performance–assessment monitoring plots showing direct measurements of  $^{90}\text{Sr}$  concentration, and  $^{90}\text{Sr}$  equivalents (i.e., total beta radiostrontium and scaled gross beta) are presented in Figure 4.1 through Figure 4.4 for the four compliance monitoring wells (199-N-122, -123, -146, and -147) and in Figure 4.5 through Figure 4.8 for two Hanford-inclusive (199-N-142 and -145) and two Ringold-only (199-N-160 and -164) injection wells, respectively. Note that the performance plots include the full data record encompassing both low-concentration treatments in 2006 and 2007 and the high-concentration treatments in June through July 2008. Injection start times, as indicated by vertical lines on each of the plots, are included for both the injection well and adjacent injection wells to show any potential impact from adjacent injection operations. The minimum and maximum baseline range in  $^{90}\text{Sr}$  concentration was determined for each injection well based on an analysis described by Williams et al. (2008). Performance-assessment monitoring data for all injection, monitoring, and compliance monitoring wells, showing both  $^{90}\text{Sr}$  and calcium/phosphate trend plots, are provided in Appendix B.

Short-term increases in the  $^{90}\text{Sr}$  concentration associated with the injection of high-ionic-strength solutions during the high-concentration treatments were generally comparable to those observed during the initial low-concentration treatments, with some wells indicating a somewhat higher degree of  $^{90}\text{Sr}$  mobilization and others indicating less mobilization. One notable exception was the high  $^{90}\text{Sr}$  concentrations (and associated calcium concentrations) observed in Ringold-only injection well 199-N-162 and nearby aquifer tube NVP2 (data provided in Appendix B).

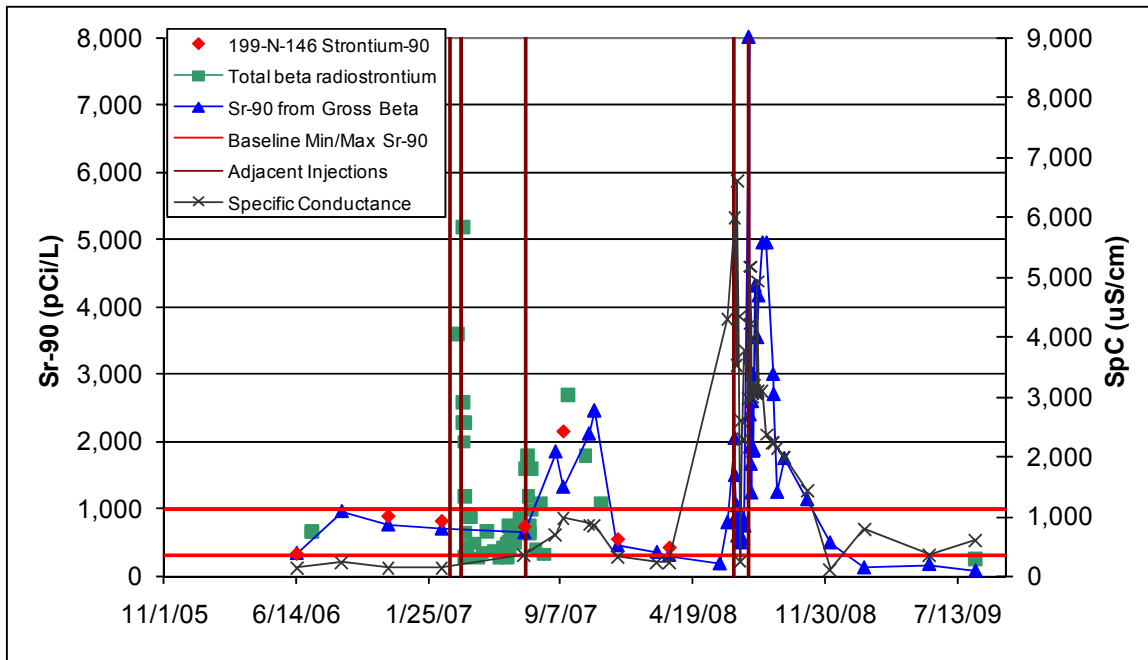
The full data record indicates a stepwise improvement in  $^{90}\text{Sr}$  sequestration performance between the low- and high-concentration injections, with concentrations at nearly all monitoring locations well below the low end of the baseline range by 1 year after the high-concentration treatment. A summary of the percent reduction in  $^{90}\text{Sr}$  concentration for all injection, compliance, and pilot test site monitoring wells is provided in Table 4.1. As indicated, most wells meet the treatability test plan objective of a 90% reduction in  $^{90}\text{Sr}$  concentration.



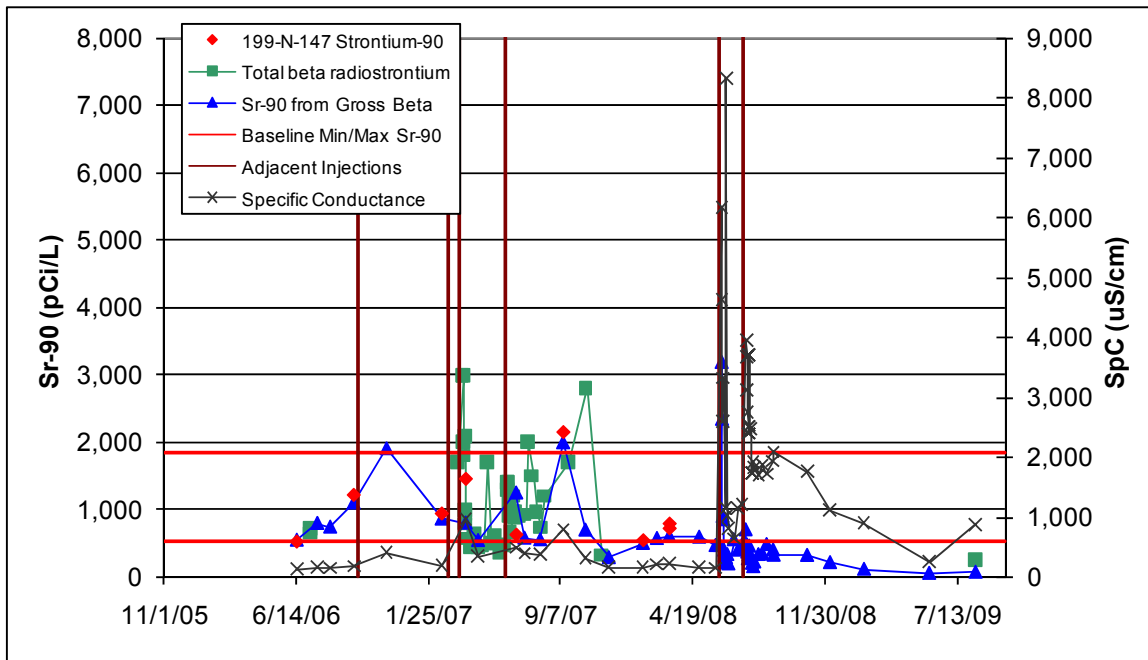
**Figure 4.1.** Performance monitoring plots for compliance monitoring well 199-N-122



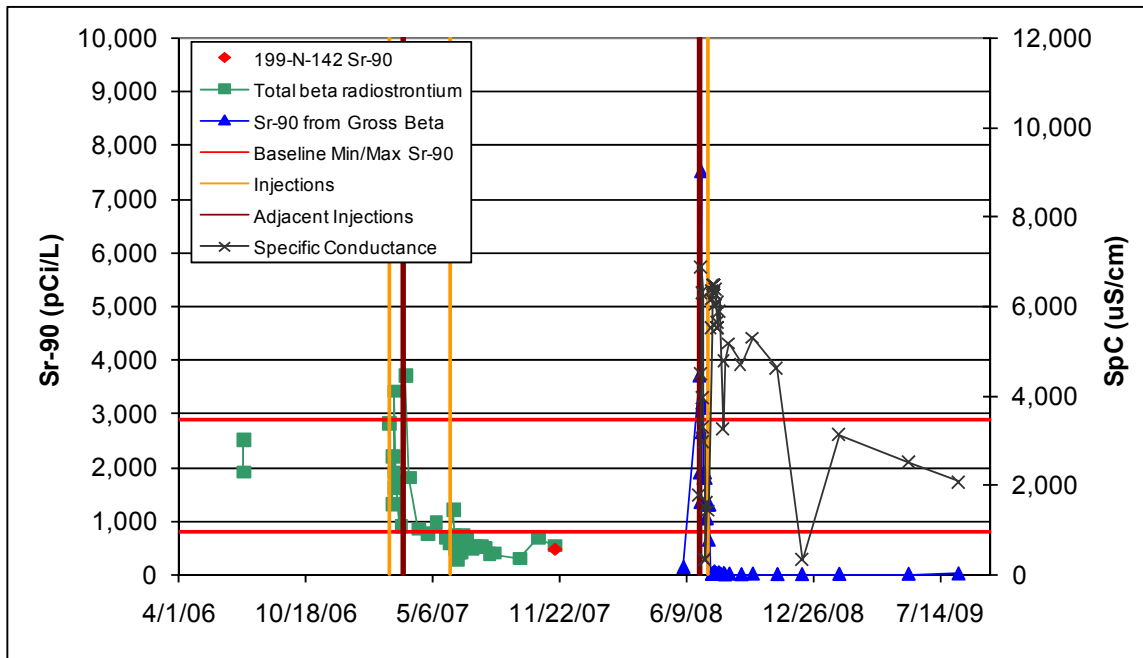
**Figure 4.2.** Performance monitoring plots for compliance monitoring well 199-N-123



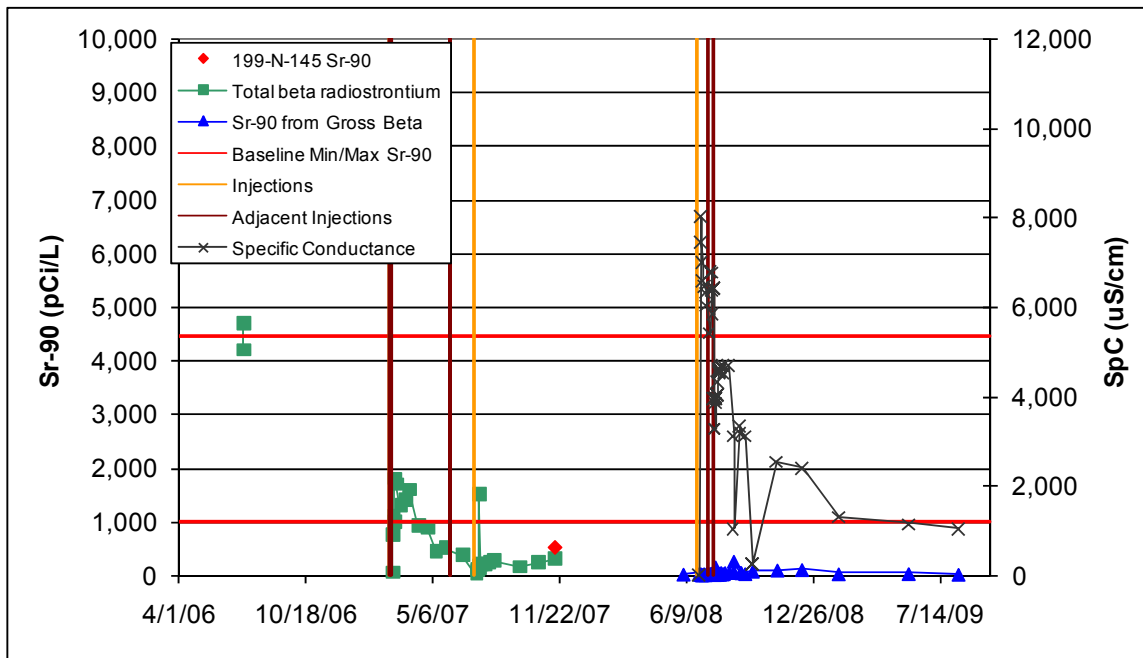
**Figure 4.3.** Performance monitoring plots for compliance monitoring well 199-N-146



**Figure 4.4.** Performance monitoring plots for compliance monitoring well 199-N-147

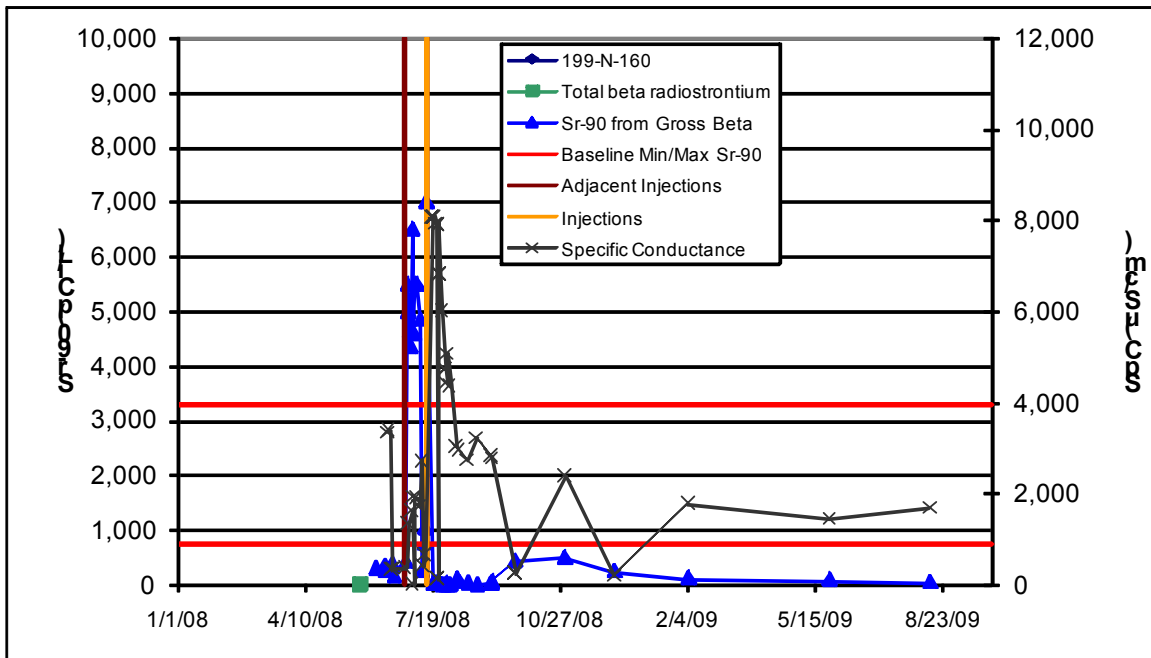


**Figure 4.5.** Performance monitoring plots for Hanford-inclusive injection well 199-N-142

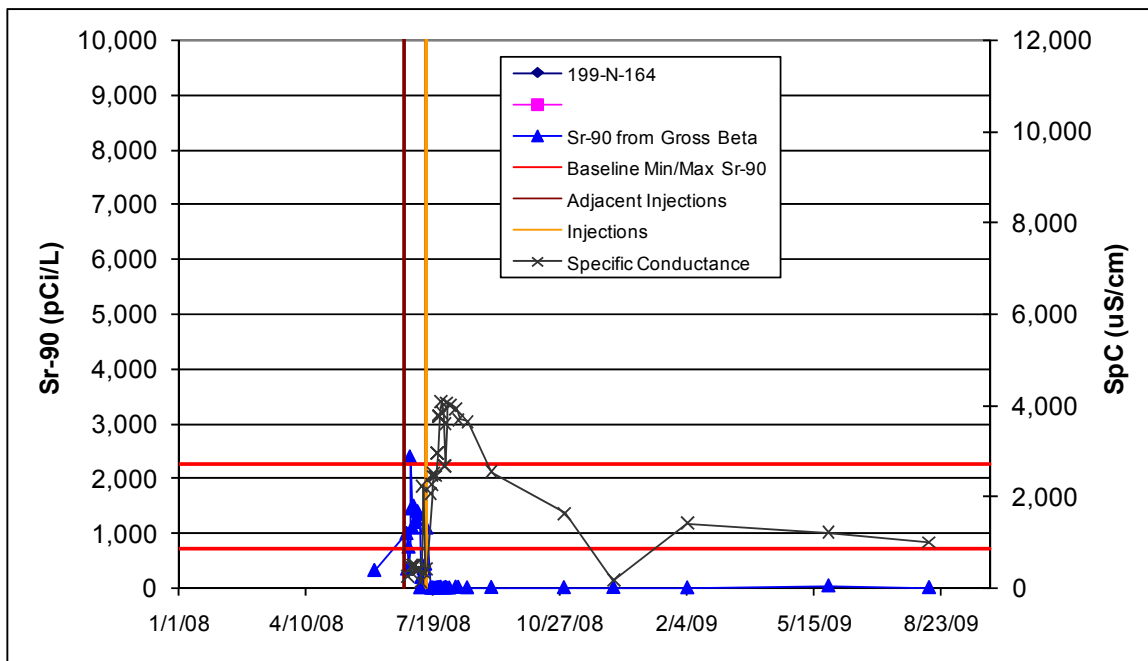


**Figure 4.6.** Performance monitoring plots for Hanford-Inclusive injection well 199-N-145





**Figure 4.7.** Performance monitoring plots for Ringold-only injection well 199-N-160



**Figure 4.8.** Performance monitoring plots for Ringold-only injection well 199-N-164

**Table 4.1.** Percent reduction in <sup>90</sup>Sr concentration

Well Name	Sample Date	<sup>90</sup> Sr Concentration (pCi/L)			Percent Reduction in <sup>90</sup> Sr Concentration	
		Min Baseline	Max Baseline	Last Observed	Min Baseline	Max Baseline
Primary Performance Assessment Monitoring Locations						
199-N-122 <sup>(a)</sup>	8/13/2009	657	4630	100	84.8%	97.8%
199-N-123 <sup>(a)</sup>	8/13/2009	689	1180	70.0	89.8%	94.1%
199-N-146 <sup>(a)</sup>	8/13/2009	318	985	85.0	73.3%	91.4%
199-N-147 <sup>(a)</sup>	8/13/2009	522	1842	65.0	87.5%	96.5%
199-N-142 <sup>(b)</sup>	8/13/2009	812	2900	23.5	97.1%	99.2%
199-N-145 <sup>(b)</sup>	8/13/2009	997	4450	20.5	97.9%	99.5%
199-N-160 <sup>(c)</sup>	8/13/2009	739	3292	50.0	93.2%	98.5%
199-N-164 <sup>(c)</sup>	8/13/2009	712	2262	11.0	98.5%	99.5%
199-N-128(P-3-R) <sup>(d)</sup>	8/13/2009	602	1103	31.5	94.8%	97.1%
199-N-129(P-4-H) <sup>(d)</sup>	5/26/2009	602	1103	85.0	85.9%	92.3%
199-N-132(P-7-R) <sup>(d)</sup>	8/13/2009	602	1103	160.0	73.4%	85.5%
199-N-133(P-8-H) <sup>(d)</sup>	5/26/2009	602	1103	100.0	83.4%	90.9%
199-N-148(P2-1-R) <sup>(d)</sup>	8/13/2009	487	1842	21.5	95.6%	98.8%
199-N-149(P2-2-H) <sup>(d)</sup>	8/13/2009	487	1842	120	75.4%	93.5%
199-N-150(P2-4-H) <sup>(d)</sup>	5/26/2009	487	1842	60	87.7%	96.7%
199-N-151(P2-3-R) <sup>(d)</sup>	8/13/2009	487	1842	95	80.5%	94.8%
199-N-155(P2-6-H) <sup>(d)</sup>	8/13/2009	487	1842	175	64.1%	90.5%
199-N-156(P2-5-R) <sup>(d)</sup>	8/13/2009	487	1842	360	26.1%	80.5%
Secondary Performance Assessment Monitoring Locations (truncated data record)						
199-N-143 <sup>(b)</sup>	2/5/2009	715	3989	13.0	98.2%	99.7%
199-N-144 <sup>(b)</sup>	2/5/2009	1538	4306	24.0	98.4%	99.4%
199-N-136 <sup>(b)</sup>	2/4/2009	480	2134	38.0	92.1%	98.2%
199-N-137 <sup>(b)</sup>	2/4/2009	487	1842	110	77.4%	94.0%
199-N-138 <sup>(b)</sup>	2/4/2009	602	1103	20.0	96.7%	98.2%
199-N-139 <sup>(b)</sup>	2/4/2009	278	829	4.55	98.4%	99.5%
199-N-140 <sup>(b)</sup>	2/4/2009	303	925	21.0	93.1%	97.7%
199-N-141 <sup>(b)</sup>	2/4/2009	611	1624	28.0	95.4%	98.3%
199-N-159 <sup>(c)</sup>	2/4/2009	484	1988	225	53.5%	88.7%
199-N-161 <sup>(c)</sup>	2/4/2009	1268	4378	75.0	94.1%	98.3%
199-N-162 <sup>(c)</sup>	2/4/2009	1127	4148	48.0	95.7%	98.8%
199-N-163 <sup>(c)</sup>	2/5/2009	764	3445	14.5	98.1%	99.6%
(a) Compliance monitoring wells						
(b) Injection wells						
(c) Ringold-only injection wells						
(d) Pilot test site monitoring wells						

## 5.0 Summary and Conclusions

Bench- and field-scale treatability tests conducted to date using both low- and high-concentration apatite amendment formulations indicate that the most favorable formulation for field-scale deployment of the technology consists of 3.6 mM calcium, 9 mM citrate, and 40 mM phosphate. This amendment solution, which was used in all high-concentration treatments conducted in FY08, was identified as the best formulation for meeting the following objectives: 1) minimize the number of injection operations required, 2) minimize short-term increases in  $^{90}\text{Sr}$  concentrations associated with injection of high-ionic-strength solutions, and 3) keep amendment formulations well below solubility limits to reduce the potential for operational challenges associated with solution stability.

Based on design analysis and chemical arrival responses observed during low-concentration pilot-scale testing and PRB treatment operations (Williams et al. 2008) and pilot-scale field tests with the high-concentration solution, an injection volume of 120,000 gallons of apatite amendment solution was specified for each well (or Hanford/Ringold well pair). Over the downstream portion of the barrier, two separate injection operations of approximately 60,000 gallons each were required for targeted treatment of the Hanford and Ringold formations. An injection rate of 40 gpm was specified for treatment of the upstream portion of the barrier based on the injection well hydraulic performance observed during previous barrier-treatment operations and injection design analyses indicating that these rates were sufficient for effective delivery of the phosphate amendment. During treatment of the downstream portion of the barrier, an injection rate of 20 gpm was specified for both the Hanford-inclusive and Ringold-only injection wells (for a total of 40 gpm). Injections targeting the Hanford formation were performed during high-Columbia-River-stage conditions to treat as much of the Hanford formation profile as possible. It was originally thought that treatment of the Ringold-only injection wells could be conducted during any river-stage condition; amendment arrival response data collected during the high-concentration injections indicates that treatment of the Ringold Formation is most effective when conducted in Ringold-only injection wells at low-Columbia-River stage. This approach should be used in all future injections.

Design criteria for the high-concentration injection operations were based on 1) amendment volume and mass injected, 2) amendment arrival at adjacent wells, 3) water-level elevation during treatment, and 4) injection rate limitations associated with well plugging. An evaluation of compliance with these injection design criteria was used to assess operational performance and identify candidate wells for supplemental treatment. Injection design criteria were not fully met at 8 of the 16 injection well locations, with the primary deficiency at 4 of 8 locations being the limited vertical extent of Hanford formation treatment due to low-river-stage conditions during the injection. Wells whose extent of treatment did not meet design criteria should be considered for retreatment, or at a minimum, be placed on a watch list to identify premature  $^{90}\text{Sr}$  breakthrough in a timely manner. Although injection design criteria were not fully met at a significant number of well locations, aqueous performance-assessment monitoring data collected to date indicate good barrier performance.

Short-term increases in  $^{90}\text{Sr}$  concentration associated with the injection of high-ionic-strength solutions during the high-concentration treatments were generally comparable to or less than those observed during the initial low-concentration treatment. The full data record indicates a stepwise improvement in  $^{90}\text{Sr}$  sequestration performance between the low- and high-concentration injections. The average reduction in  $^{90}\text{Sr}$  concentrations at the four compliance monitoring locations was 95% relative to the high end of the baseline range and 84% relative to the low end of the baseline range, indicating that the performance objective specified in the treatability test plan (90% reduction in  $^{90}\text{Sr}$  concentration) was met within 1 year of high-concentration treatment.

An evaluation based on sediment core samples collected in November 2009, approximately 1 year after the high-concentration treatments, will be used to quantify the amount of apatite formation resulting from the sequential low- followed by high-concentration treatments performed to date (i.e., an initial 1.0 mM Ca, 2.5 mM citrate, and 10 mM  $\text{PO}_4$  formulation followed by 3.6 mM Ca, 9 mM citrate, and 40 mM  $\text{PO}_4$ ). Based on the results, a determination will be made regarding the total volume of amendment solution, and thus the number of treatments, that will be required during full-scale deployment of the technology. If multiple high-concentration treatments are required, the timing of these injections must be considered. Development of the final injection approach for full-scale deployment of the PRB technology will consider the following design criteria: 1) emplace sufficient apatite to meet long-term remedial objectives, 2) limit permeability reduction by emplacing only the apatite content needed to meet remedial objectives, 3) limit short-term increases in  $^{90}\text{Sr}$  concentration associated with the injection of high-ionic-strength solutions, and 4) implementation cost.

The objective of the field treatability testing, as stated in the treatability test plan (DOE/RL 2006), is to address the following:

- Will apatite precipitate in the target zone?
- Does the apatite result in reducing  $^{90}\text{Sr}$  in groundwater?
- Given a fixed well spacing of 9.1 m (30 ft), what is the optimal injection volume per well for installation of a 91-m (300-ft) barrier wall?

All three test objectives have been addressed to some extent through the treatability test activities completed to date. The first bullet was partially addressed through analysis of sediment samples collected from coreholes within the treatment zone (Szecsody 2009) and performance-assessment groundwater monitoring following the low-concentration Ca-citrate- $\text{PO}_4$  injections (Williams et al. 2008). An evaluation based on sediment core samples planned for collection in November 2009, after the high-concentration injections, will provide a more definitive assessment of apatite formation and emplaced treatment capacity. The results will be documented in the treatability test final report scheduled for completion in FY10.

Reductions in  $^{90}\text{Sr}$  concentrations in groundwater (bullet 2 above) have been indicated by performance-assessment monitoring results after low-concentration treatments, and to a larger extent after high-concentration treatments. The treatment target of a 90% reduction in  $^{90}\text{Sr}$  concentration was realized at all four compliance monitoring wells, and at a majority of the other site wells that have been routinely monitored to assess performance.

Injection volume requirements for the fixed 9.1-m (30-ft) injection well spacing (bullet 3 above), which provides for overlap between adjacent injection wells of sufficient extent to form a continuous PRB, have been determined based on amendment arrival responses observed during the low-concentration Ca-citrate-PO<sub>4</sub> injections (Williams et al. 2008) and the high-concentration injection results described in this letter report. In addition to the specified injection volumes, it was determined that installation of injection wells targeting only the lower portion of the contaminated zone (Ringold Formation only) were needed to provide effective amendment coverage over the downstream section of the PRB. It was also determined that, in addition to the requirement that Hanford formation treatments be performed during the highest Columbia-River-stage conditions (to treat the full saturated thickness), Ringold-only injection wells should be treated during low-Columbia-River-stage conditions to achieve an acceptable radial extent of treatment.



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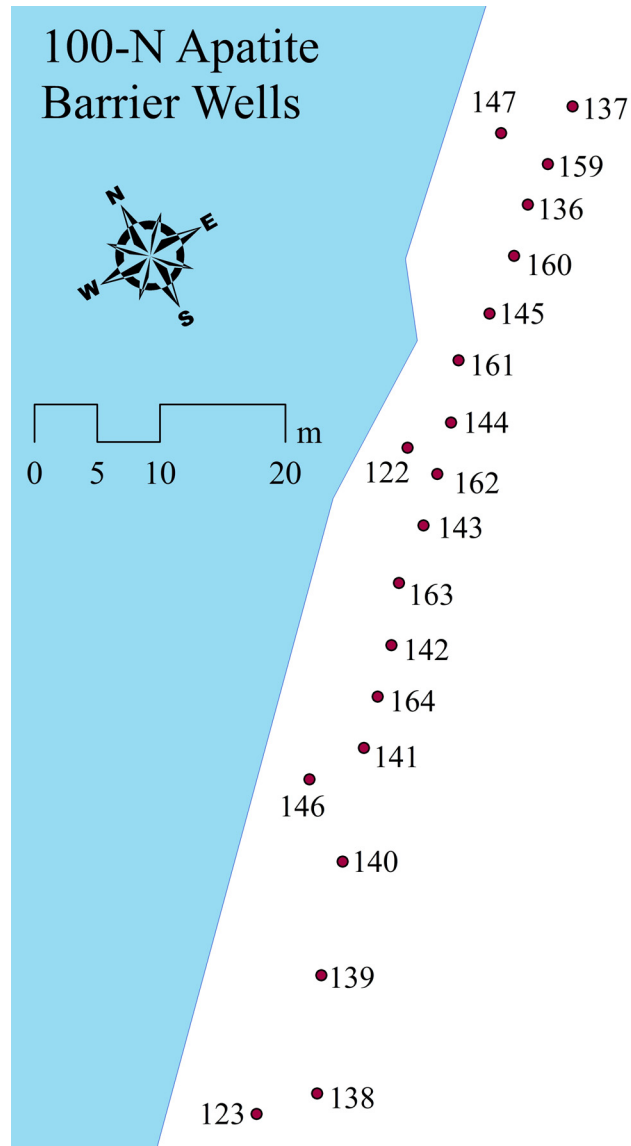
## **Appendix A**

### **Operational Performance Summary: Arrival Curves for Apatite Amendment Injections**



## Appendix A

### Operational Performance Summary: Arrival Curves for Apatite Amendment Injections



**Figure .1.** Location of injection wells for the reactive barrier

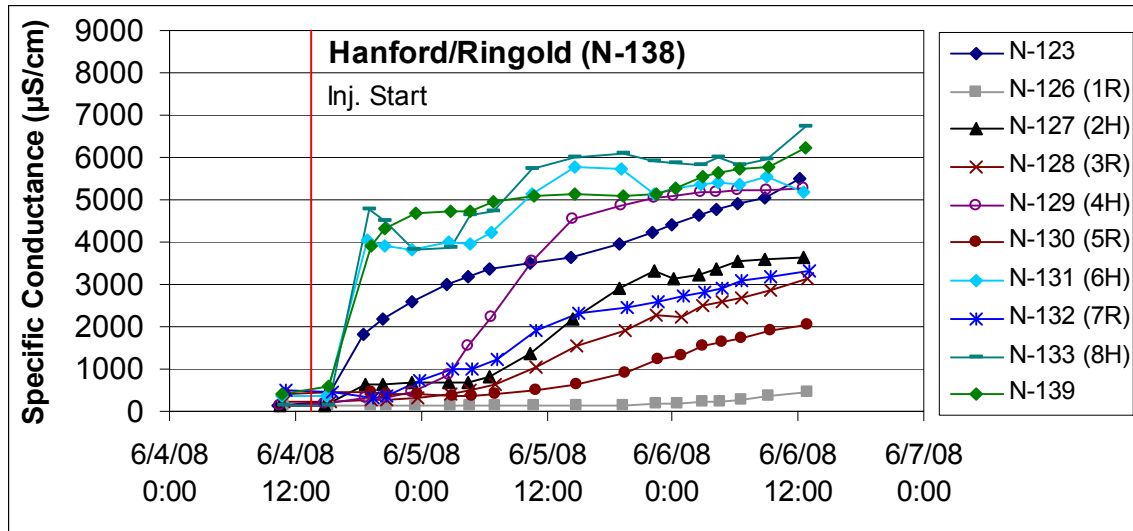


Figure A.2. Arrival curves for treatment of well 199-N-138

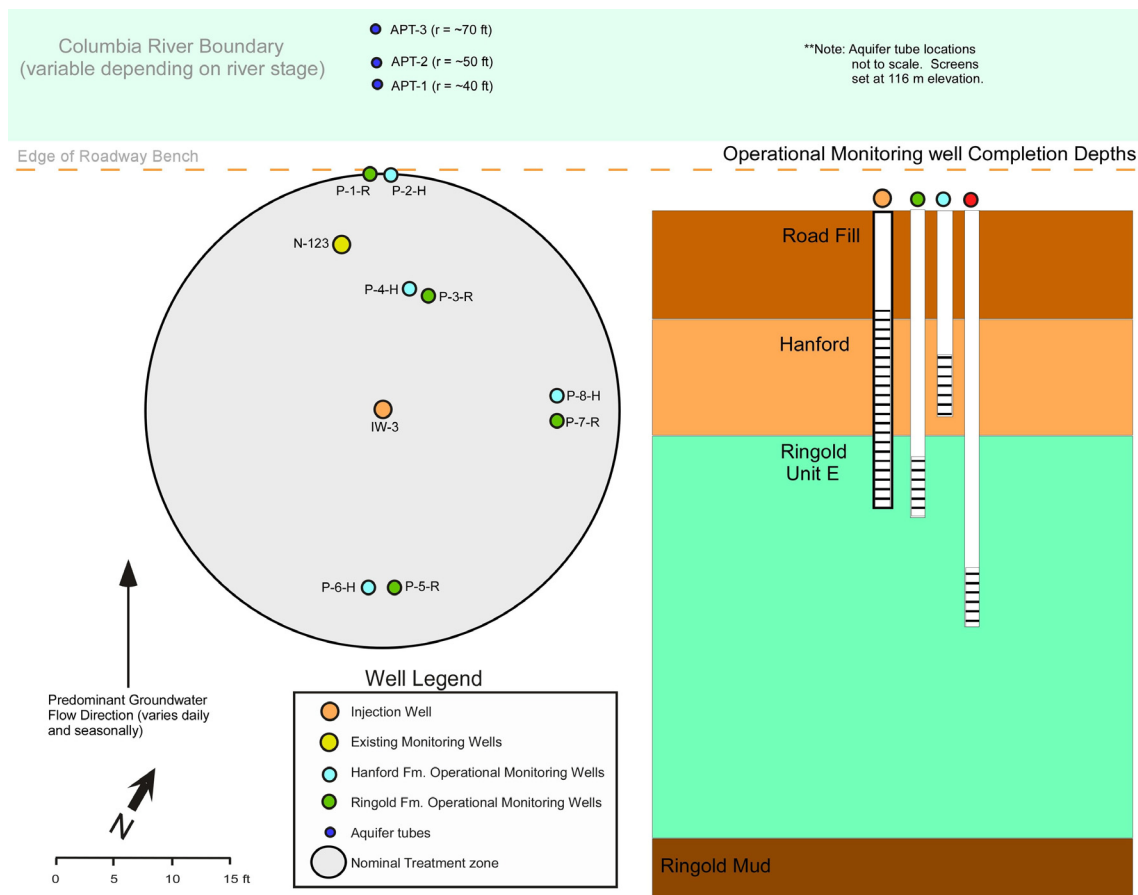


Figure A.3. Well locations at pilot test site 1

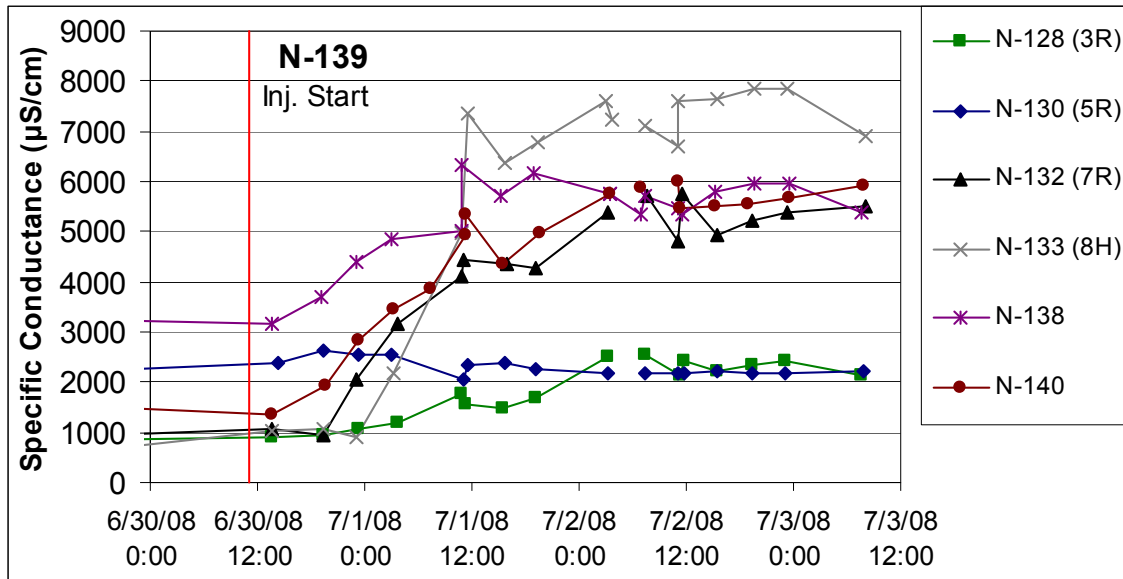


Figure A.4. Arrival curves for treatment of well 199-N-139

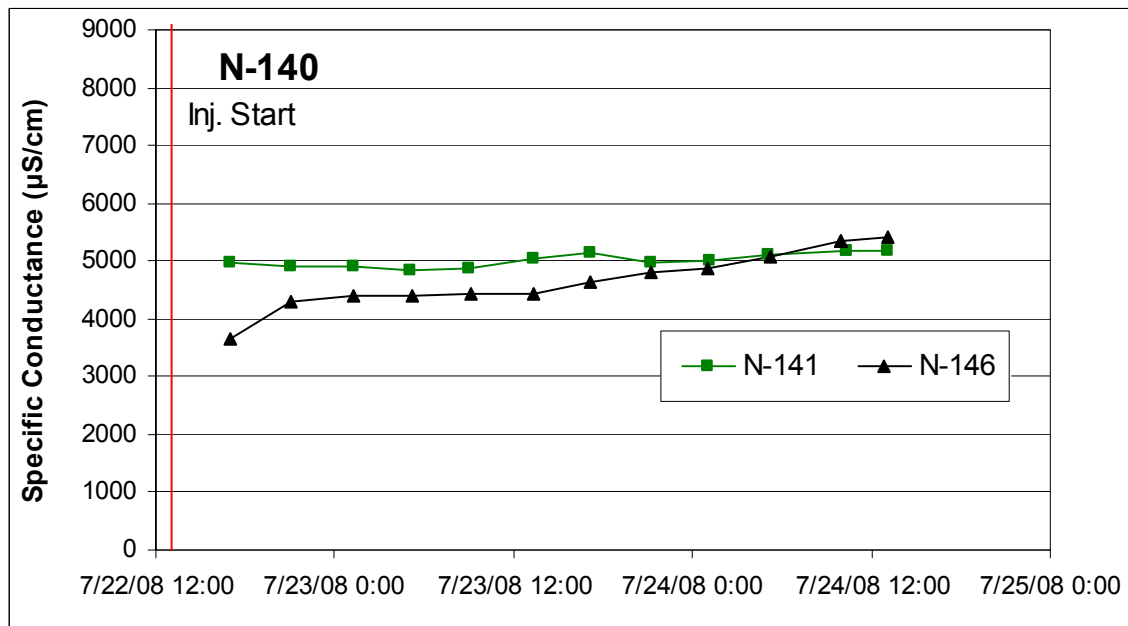
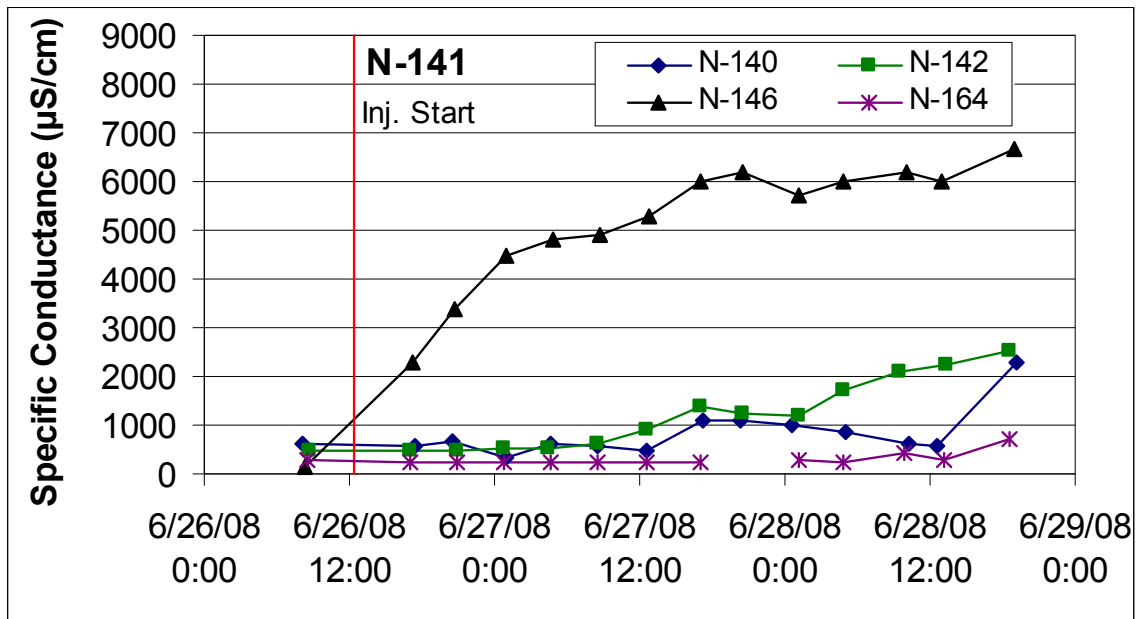
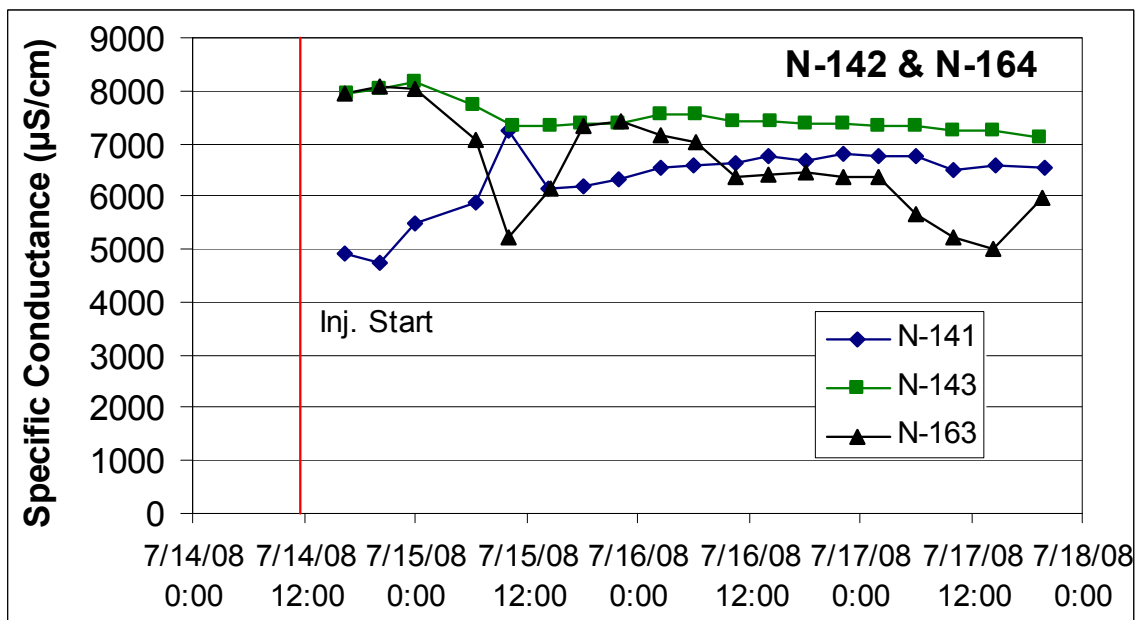


Figure A.5. Arrival curves for treatment of well 199-N-140

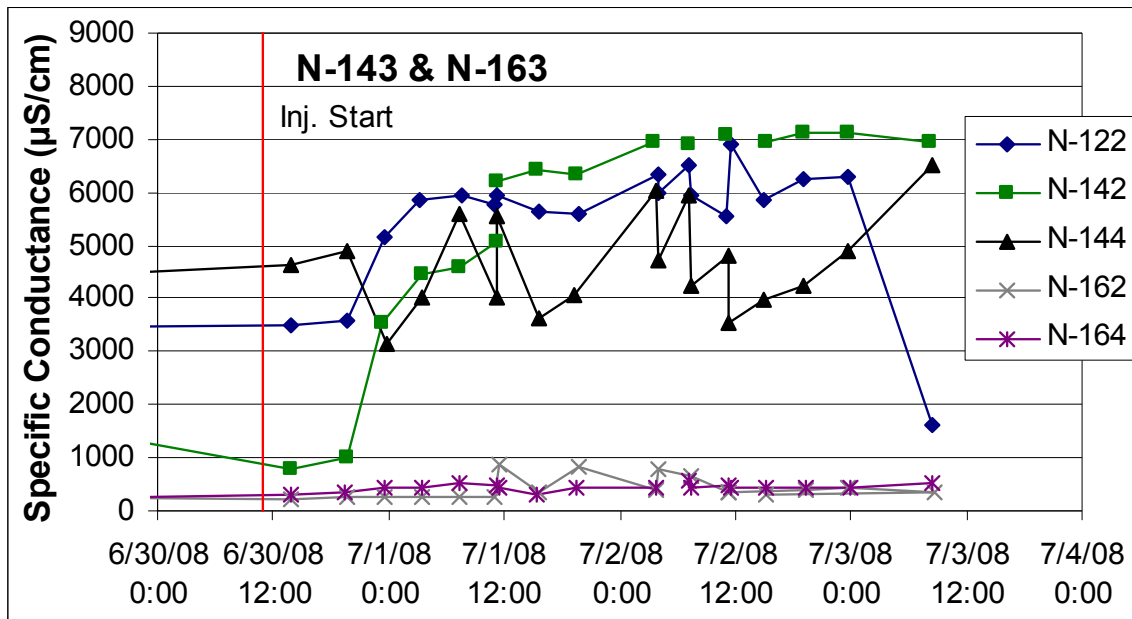


**Figure A.6.** Arrival curves for treatment of well 199-N-141

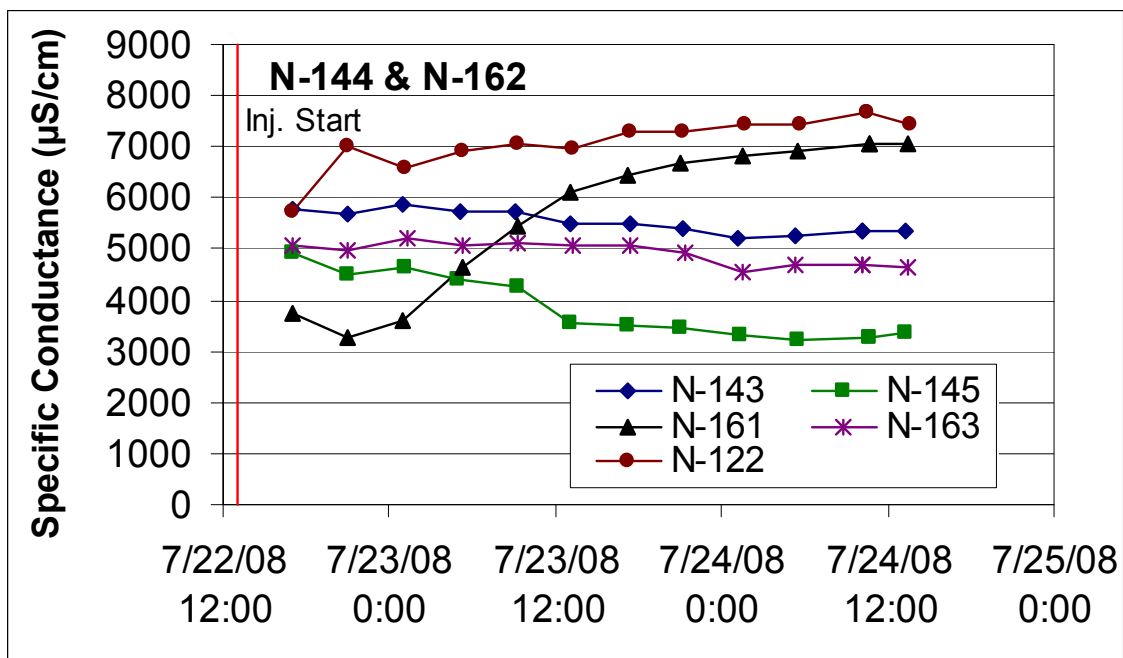


**Figure A.7.** Arrival curves for treatment of wells 199-N-142 and 199-N-164

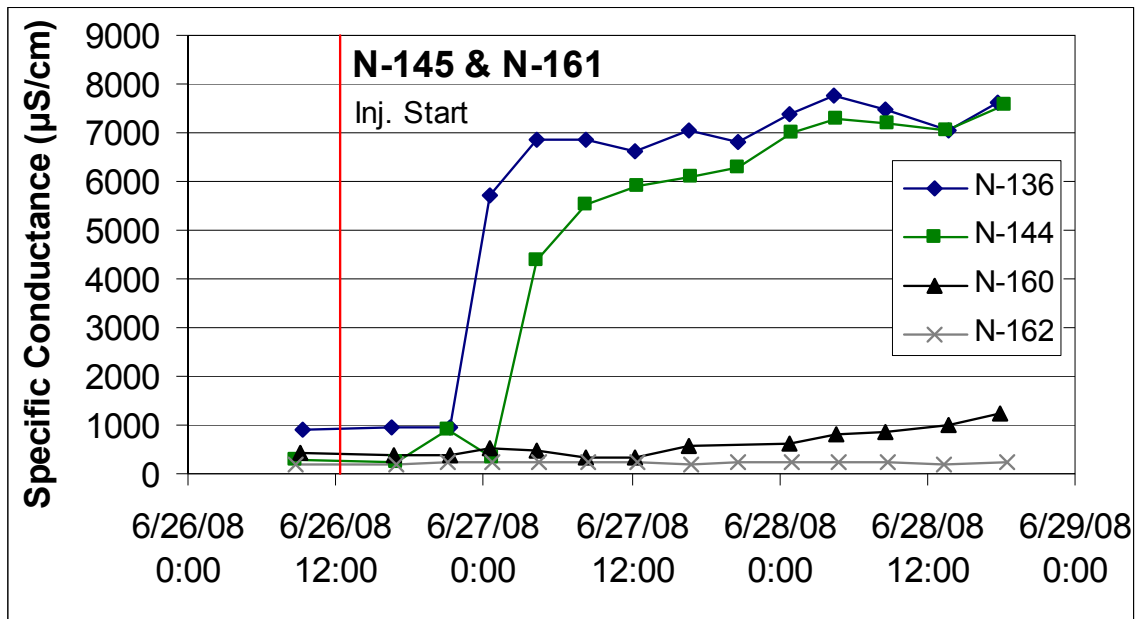




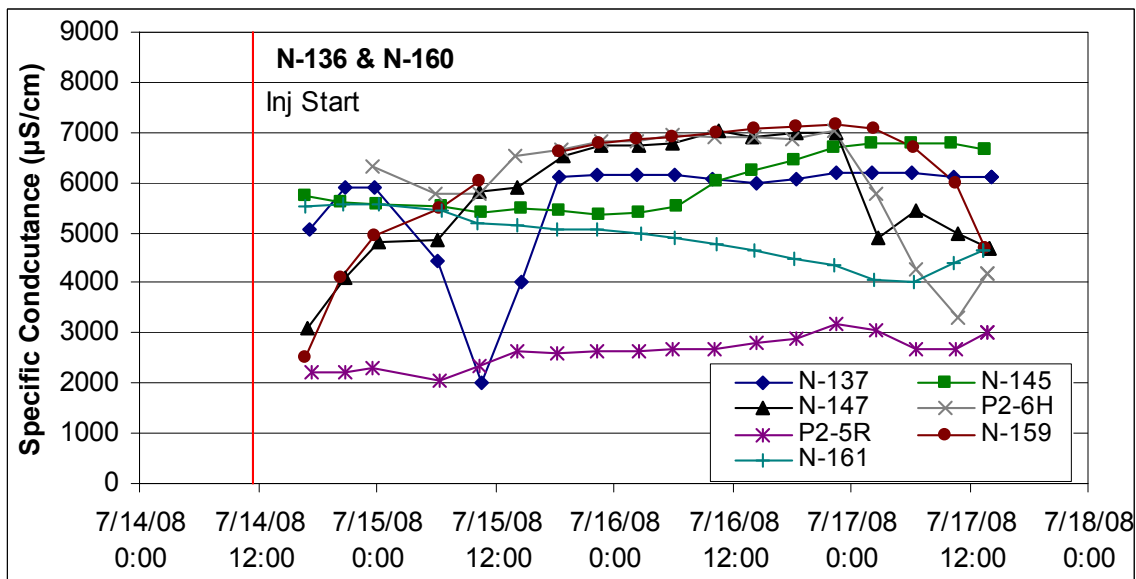
**Figure A.8.** Arrival curves for treatment of wells 199-N-143 and 199-N-163



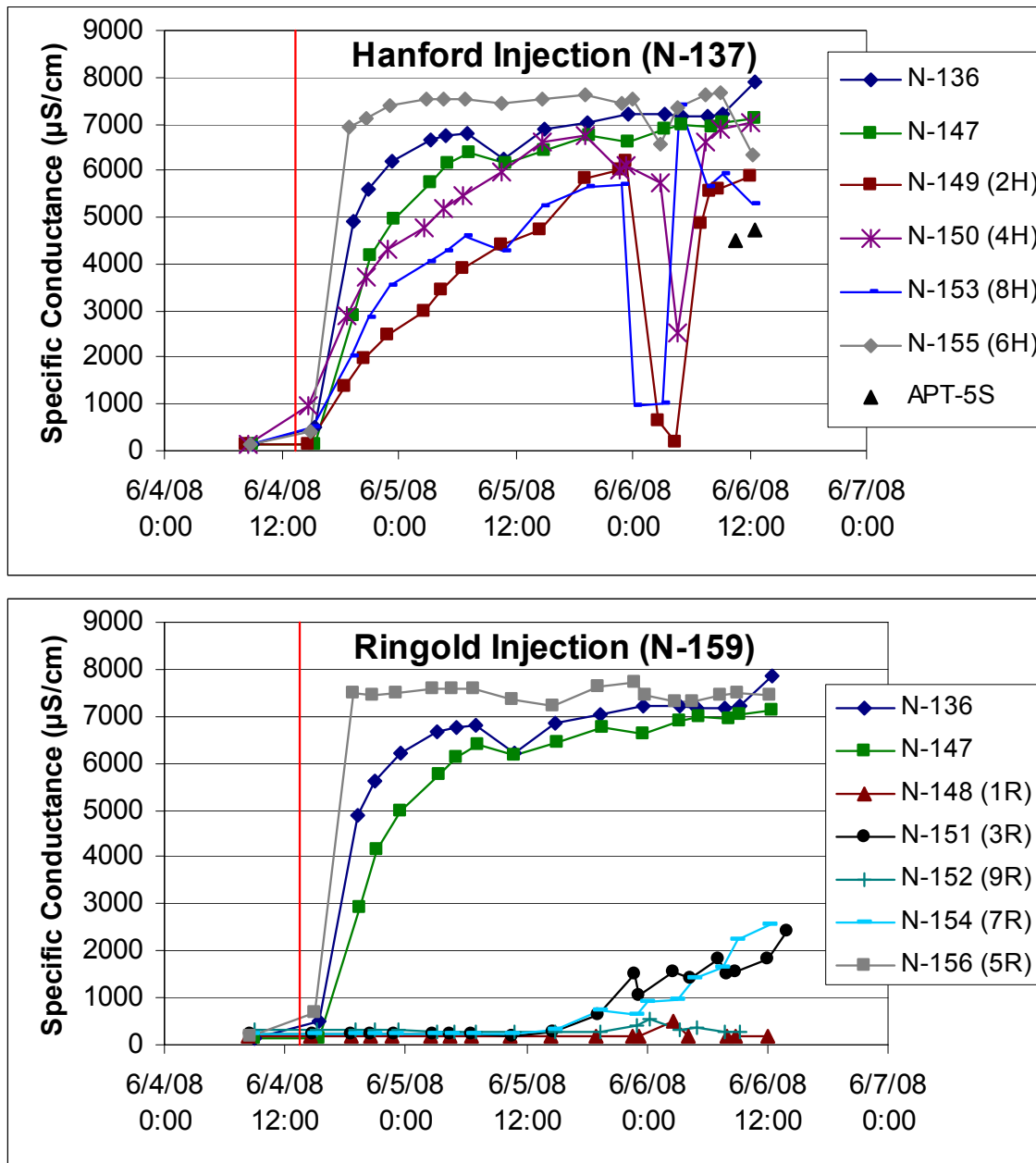
**Figure A.9.** Arrival curves for treatment of wells 199-N-144 and 199-N-162



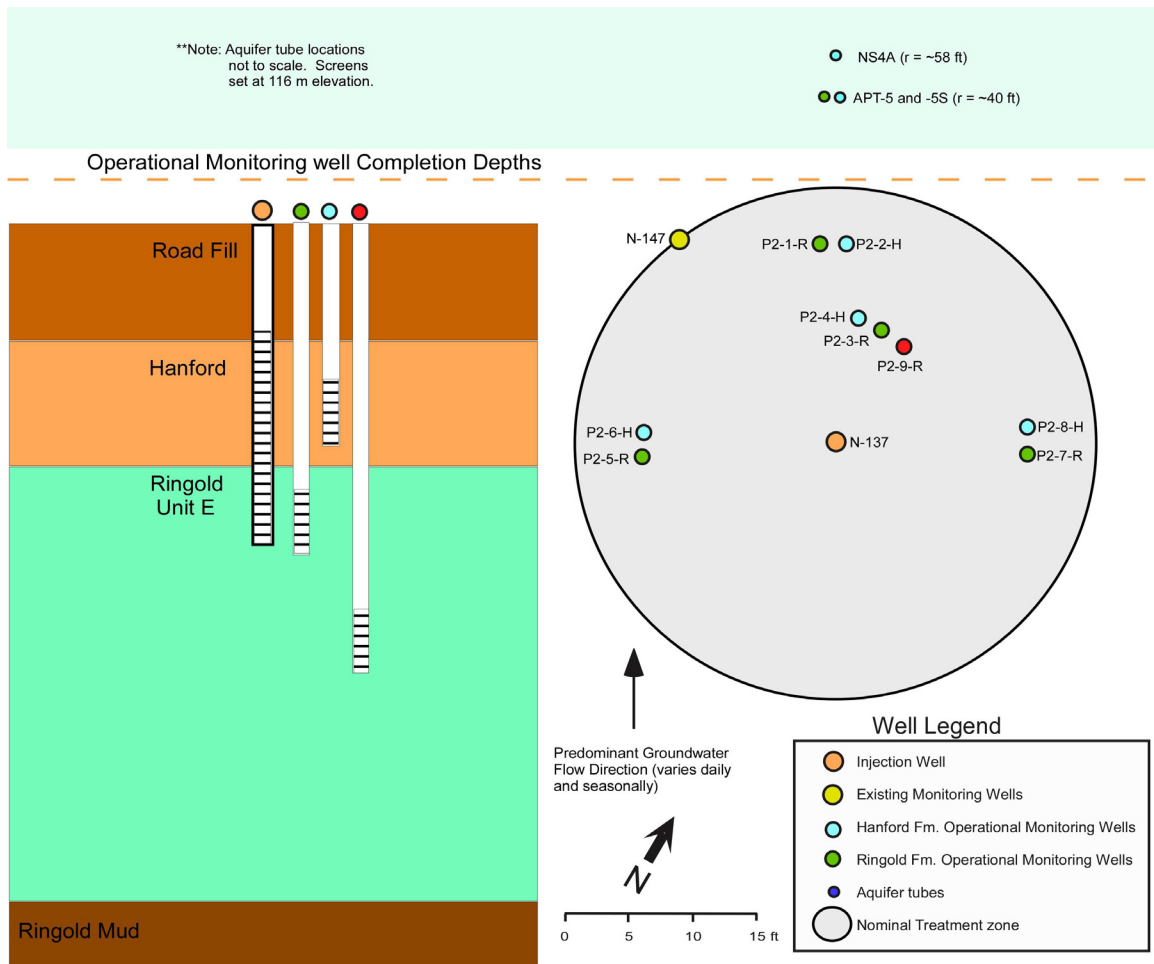
**Figure A.10.** Arrival curves for treatment of wells 199-N-145 and 199-N-161



**Figure A.11.** Arrival curves for treatment of wells 199-N-136 and 199-N-160



**Figure A.12.** Arrival curves for treatment of wells 199-N-137 and 199-N-159



**Figure A.13.** Well locations at pilot test site 2

## **Appendix B**

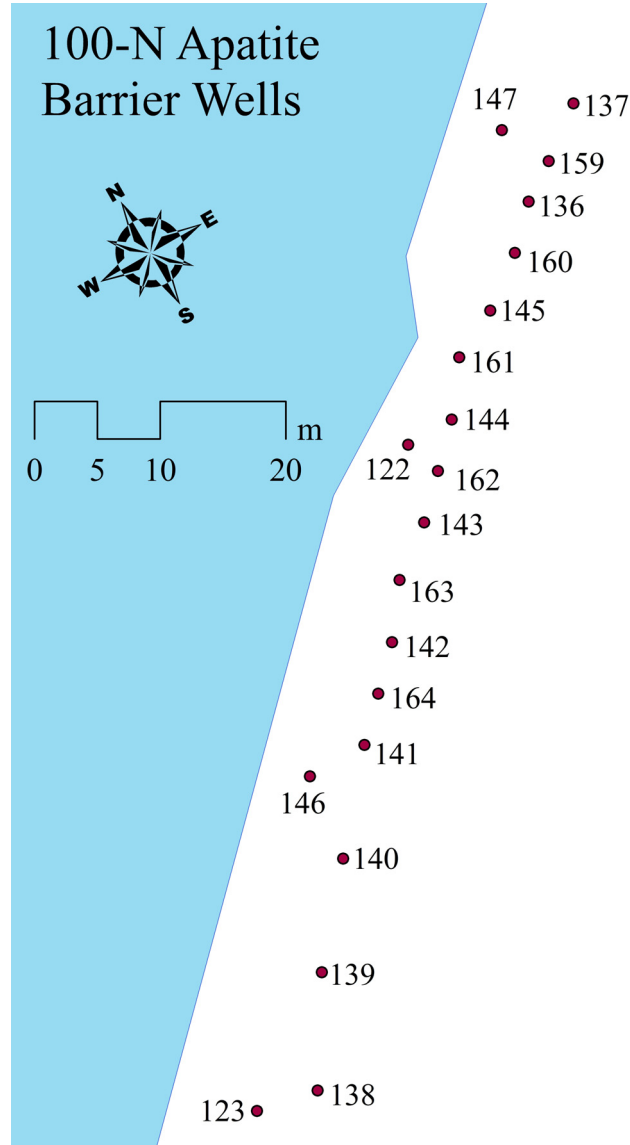
### **Performance Plots for All Groundwater Monitoring Points Associated with the Apatite Barrier**



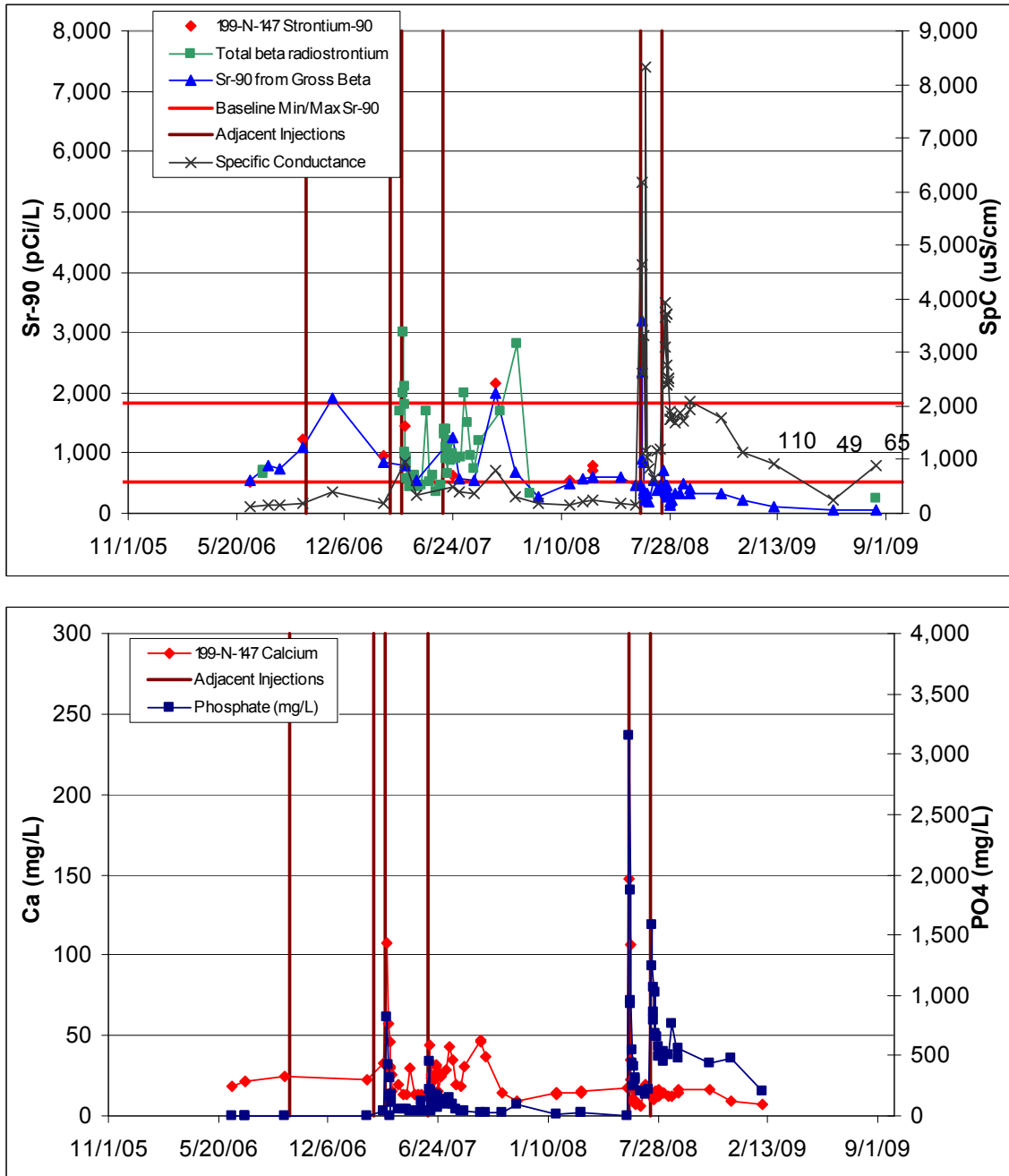
## Appendix B

### Performance Plots for All Groundwater Monitoring Points Associated with the Apatite Barrier

#### B.1 Compliance Wells

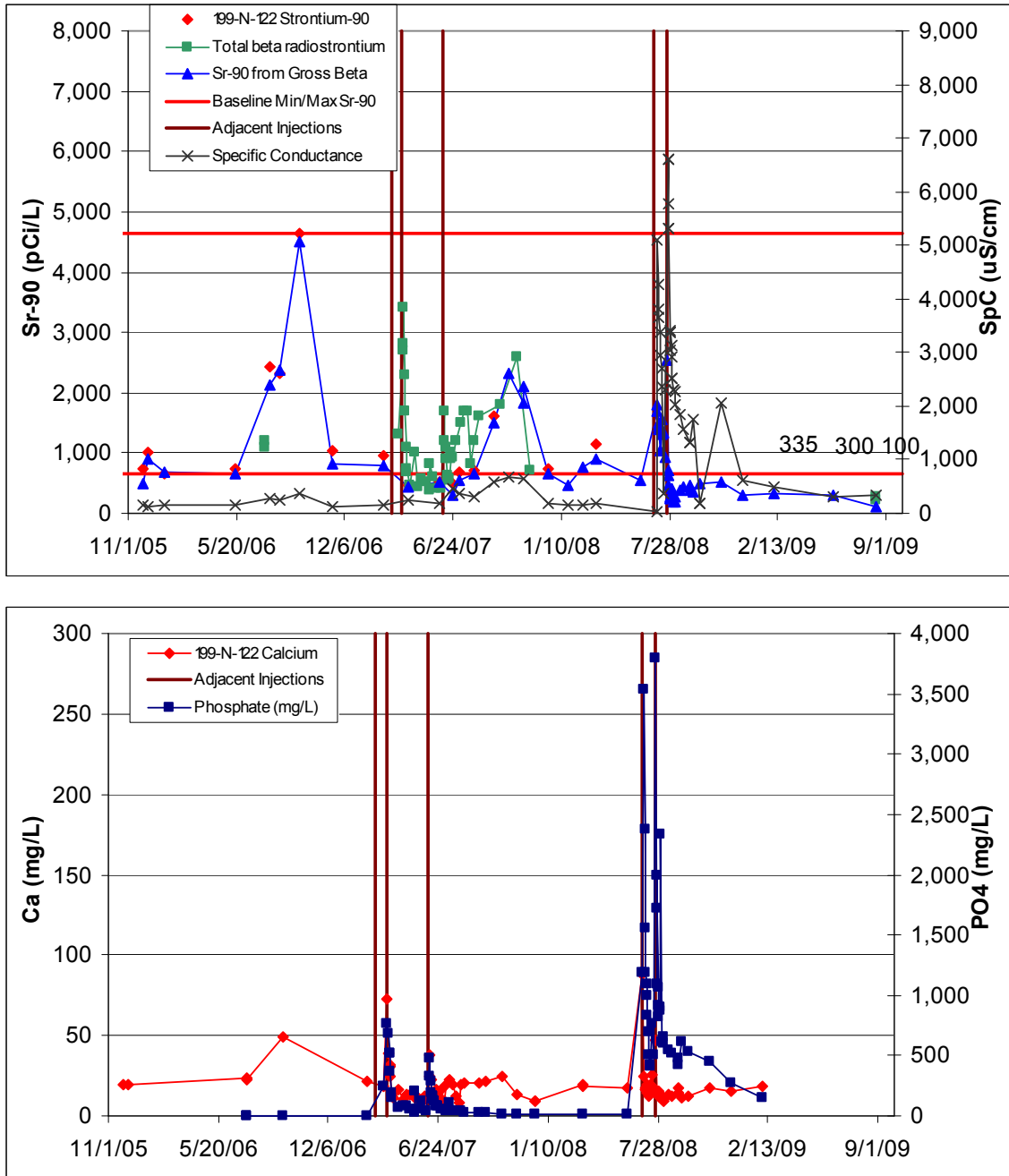


**Figure B.1.** Location of compliance monitoring wells along the reactive barrier

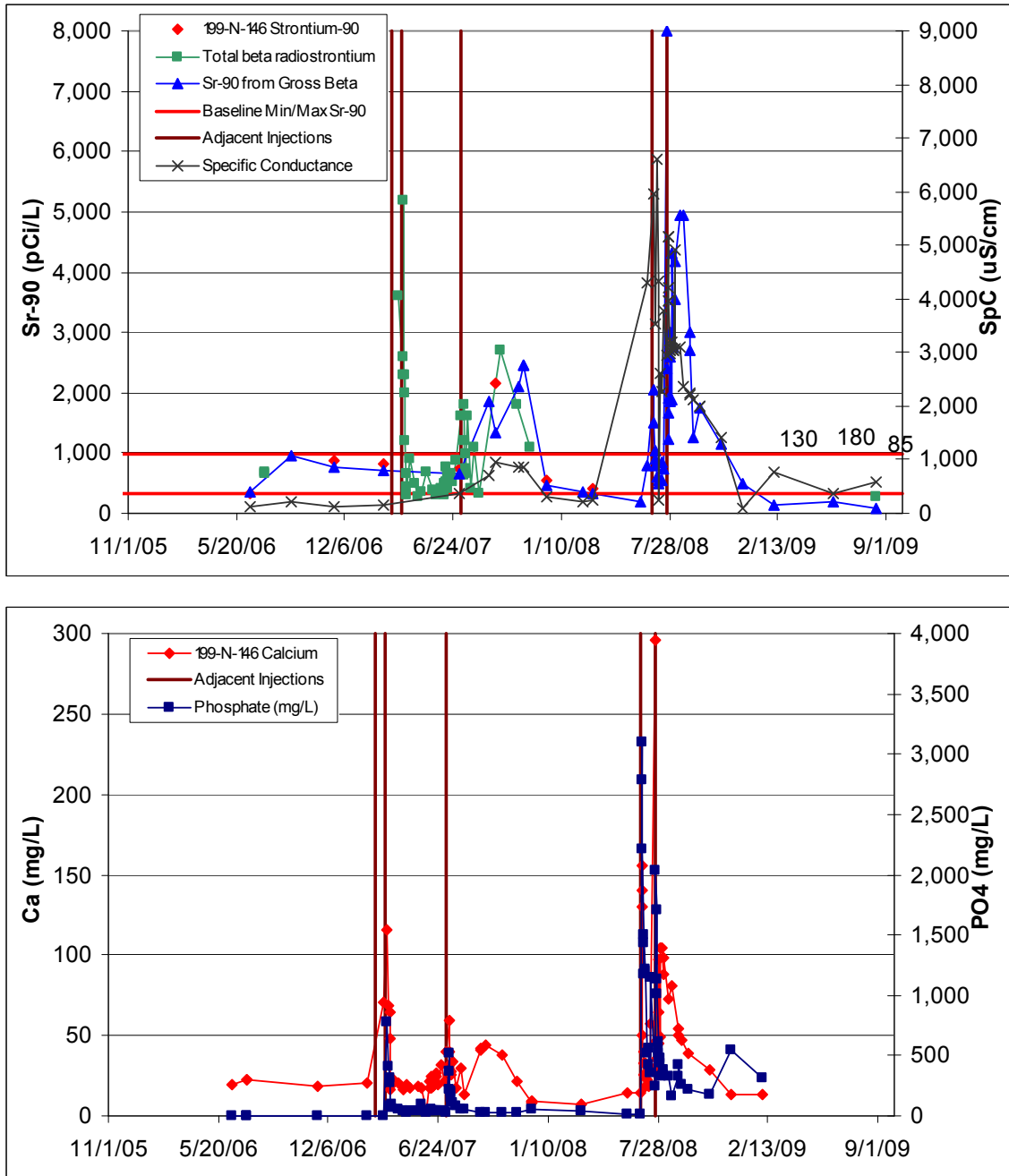


**Figure B.2.** Performance plots for well 199-N-147

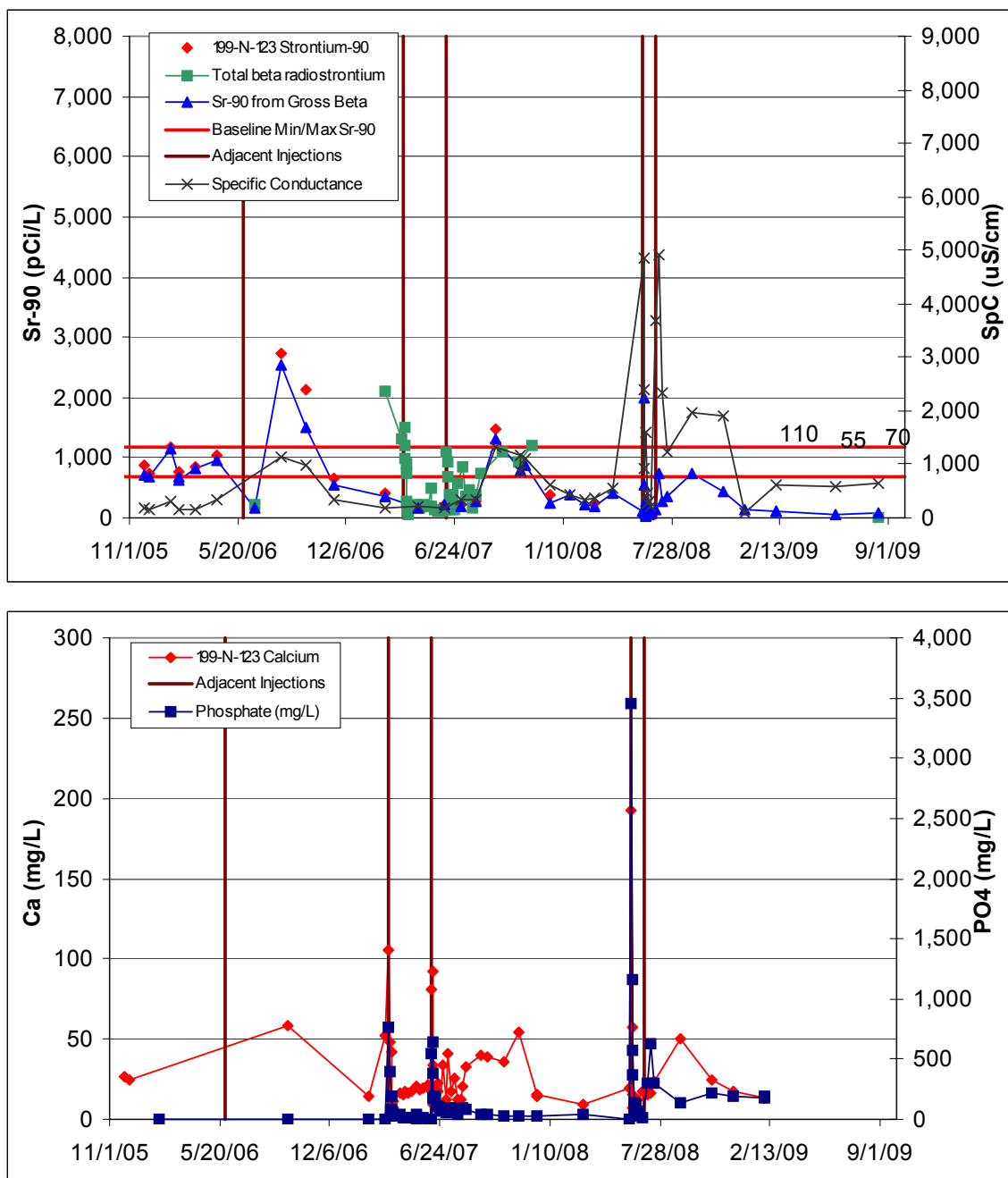




**Figure B.3.** Performance plots for well 199-N-122

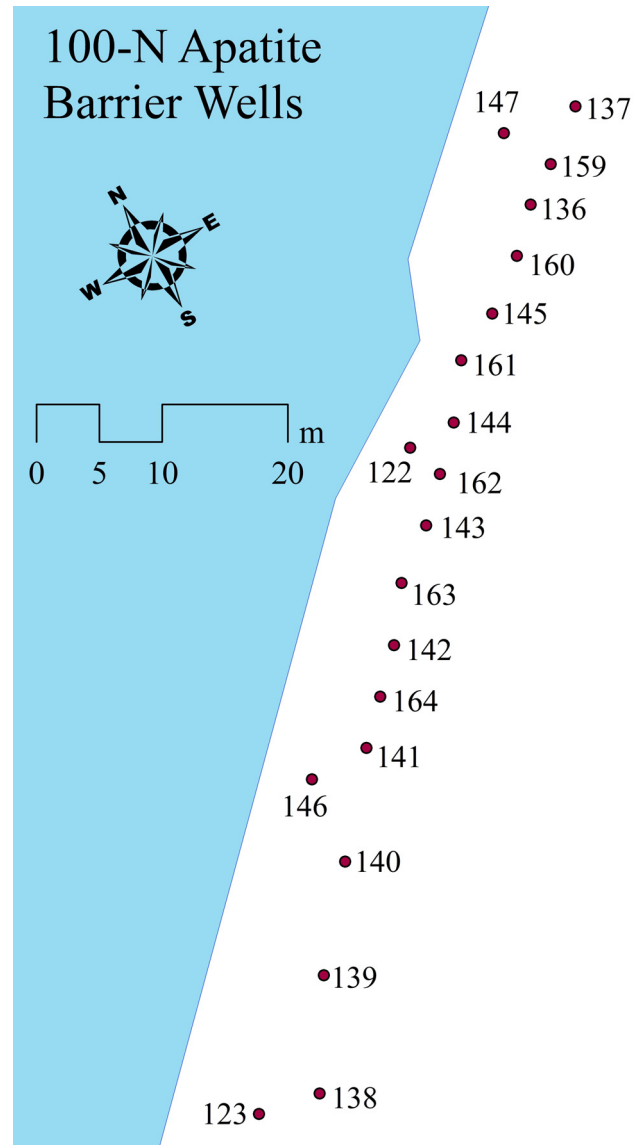


**Figure B.4.** Performance plots for well 199-N-146

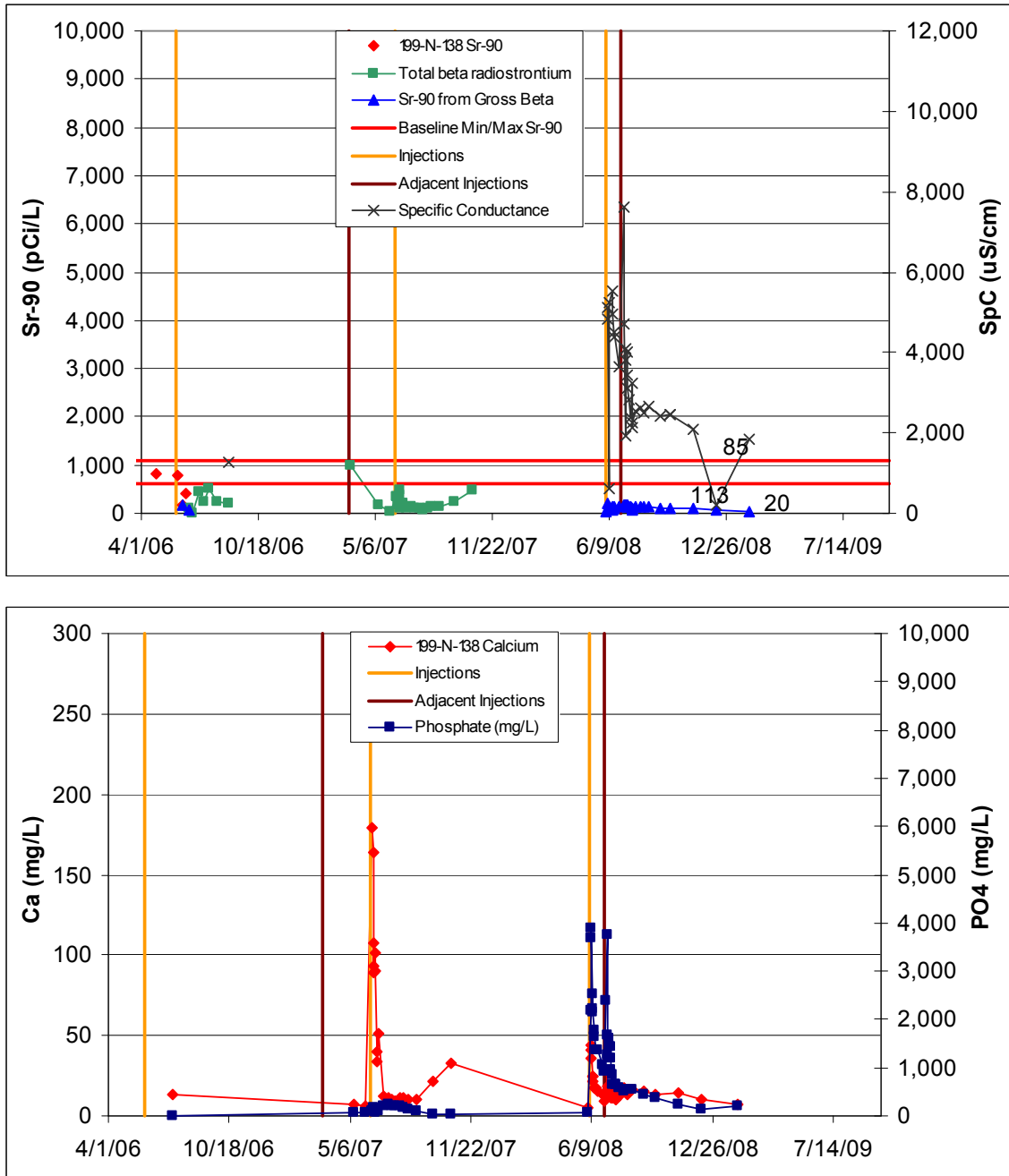


**Figure B.5.** Performance plots for well 199-N-123

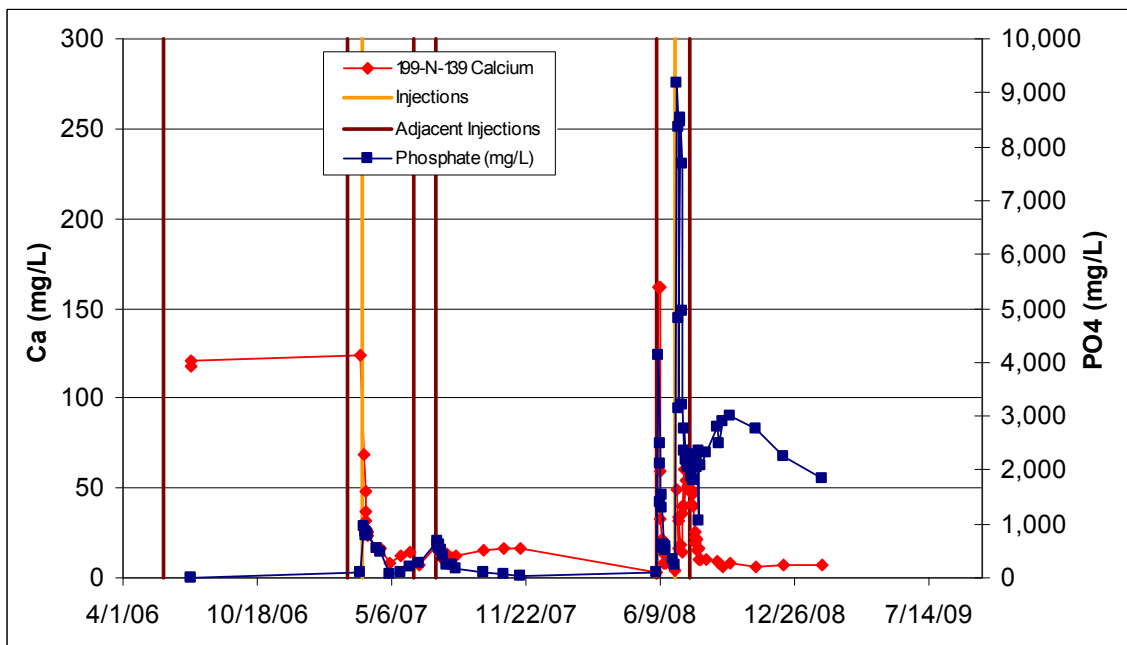
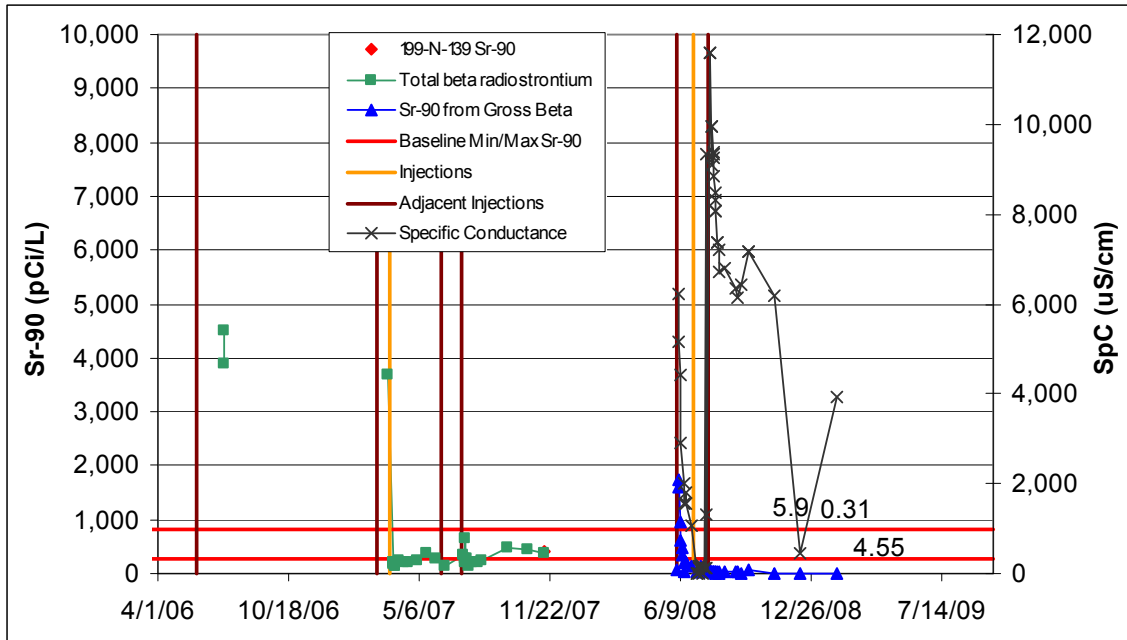
## B.2 Injection Wells



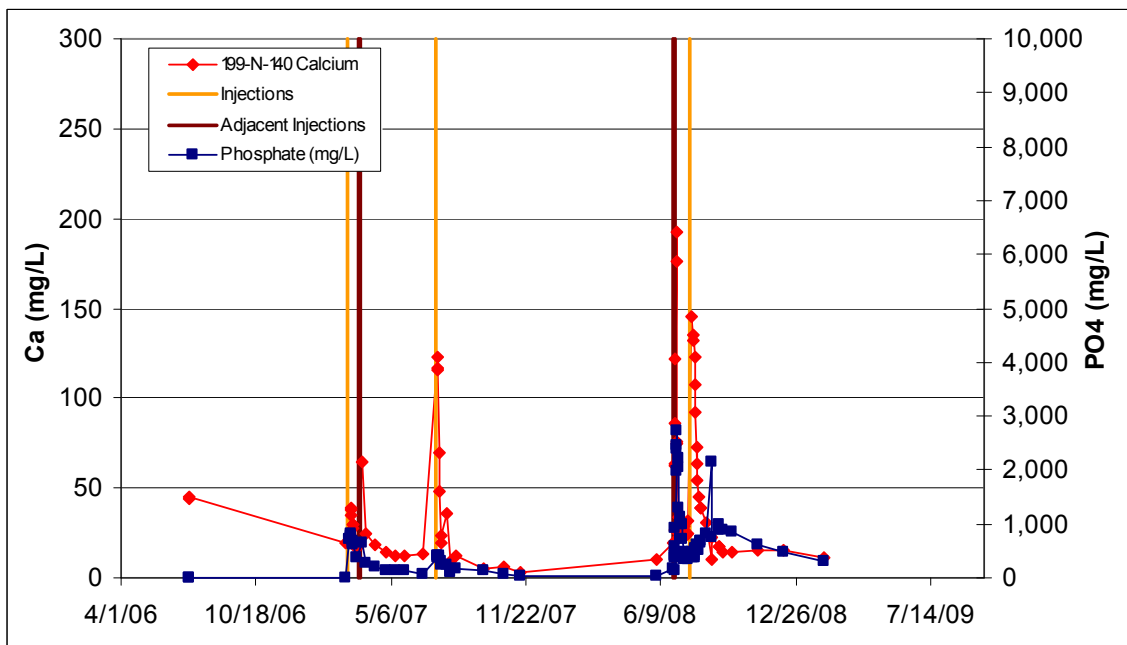
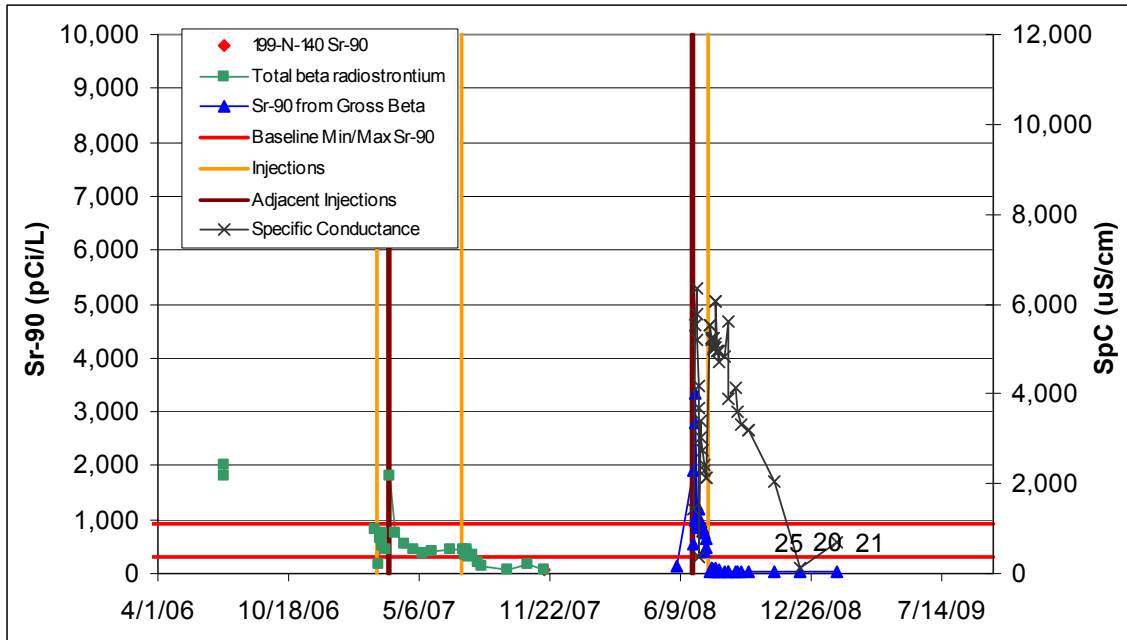
**Figure B.6.** Location of injection wells for the reactive barrier



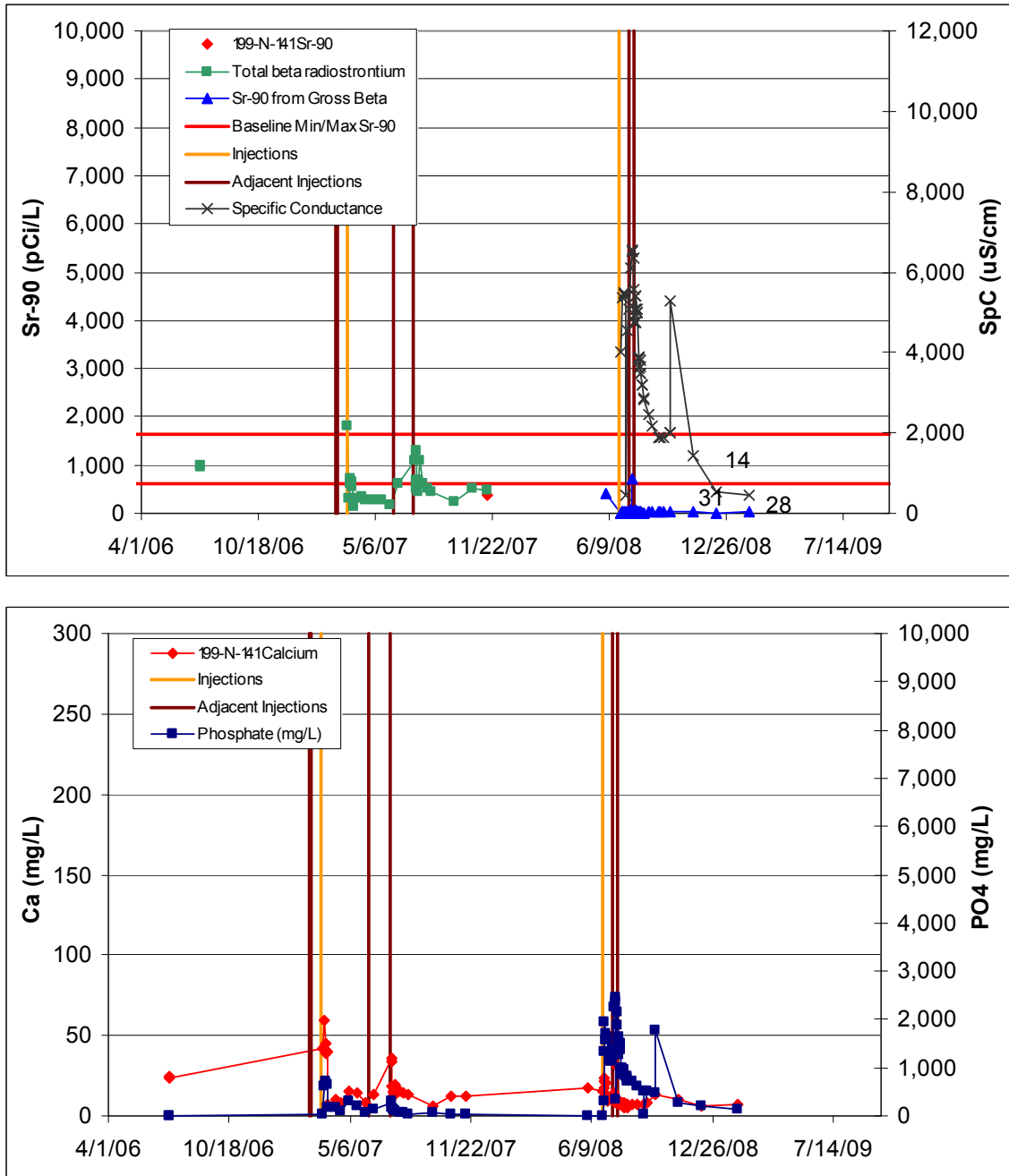
**Figure B.7.** Performance plots for well 199-N-138



**Figure B.8.** Performance plots for well 199-N-139



**Figure B.9.** Performance plots for well 199-N-140



**Figure B.10.** Performance plots for well 199-N-141



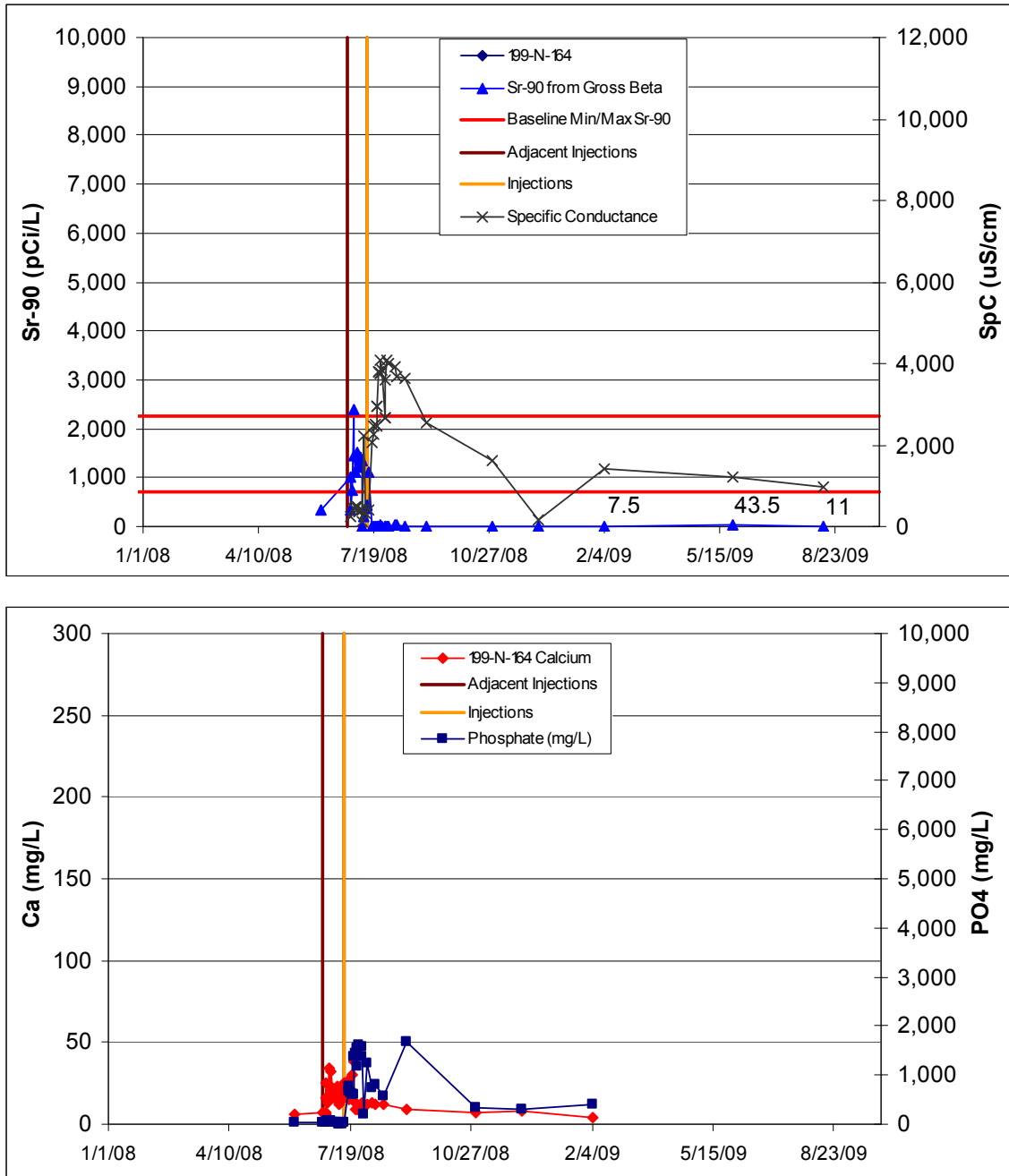


Figure B.11. Performance plots for well 199-N-164

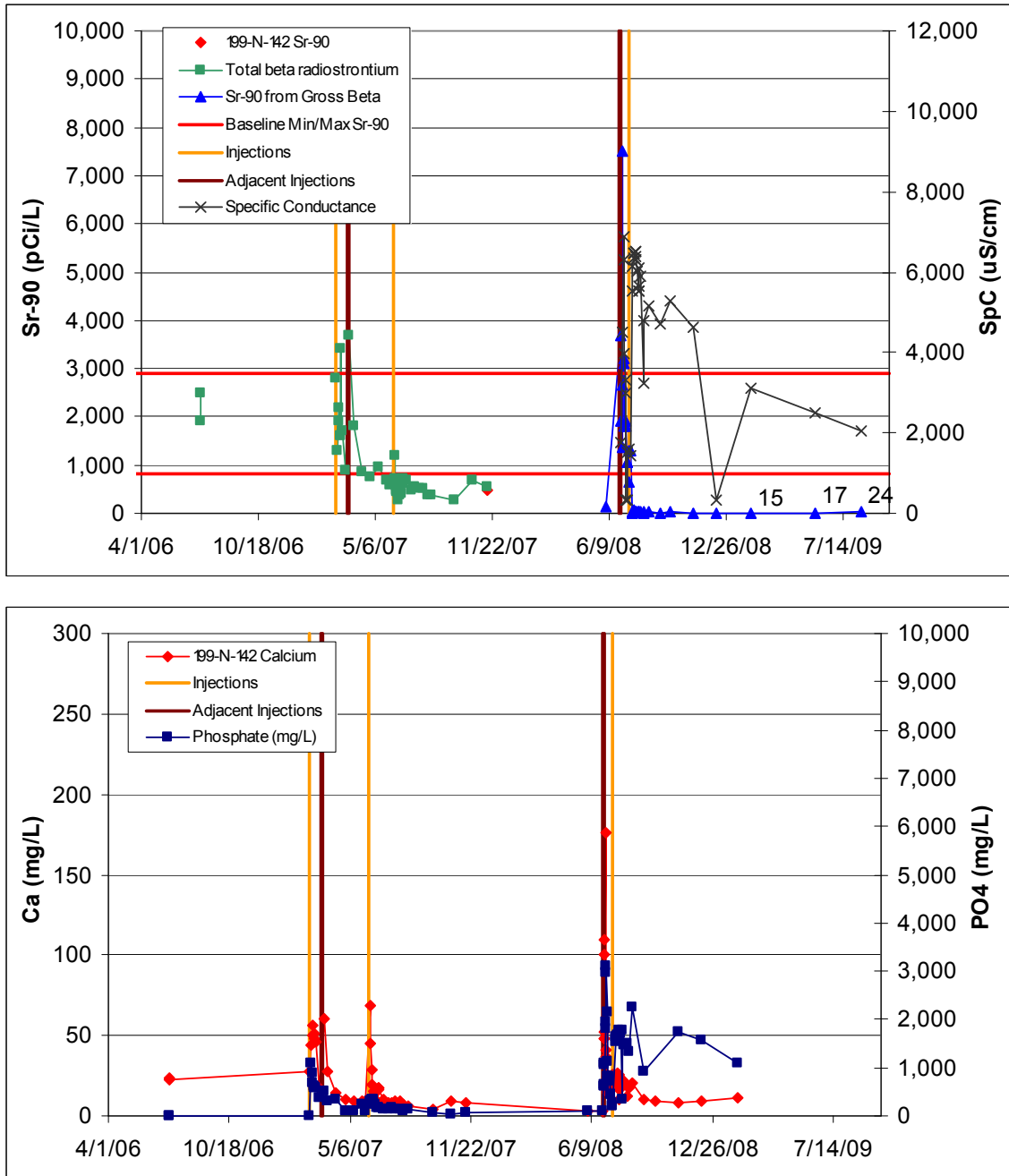


Figure B.12. Performance plots for well 199-N-142

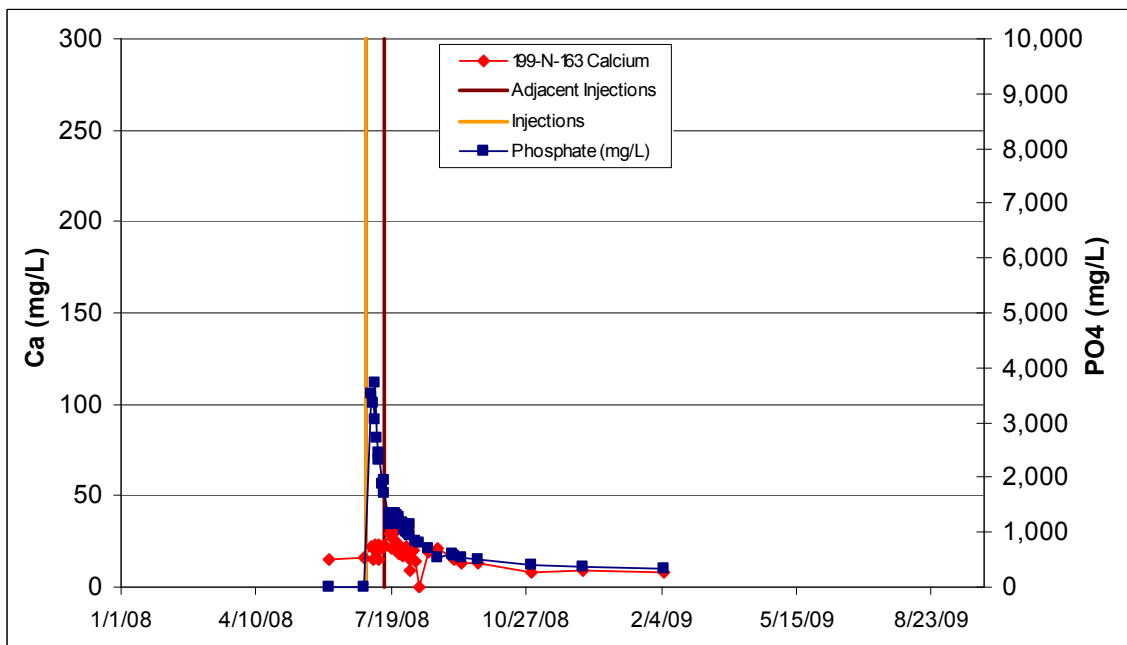
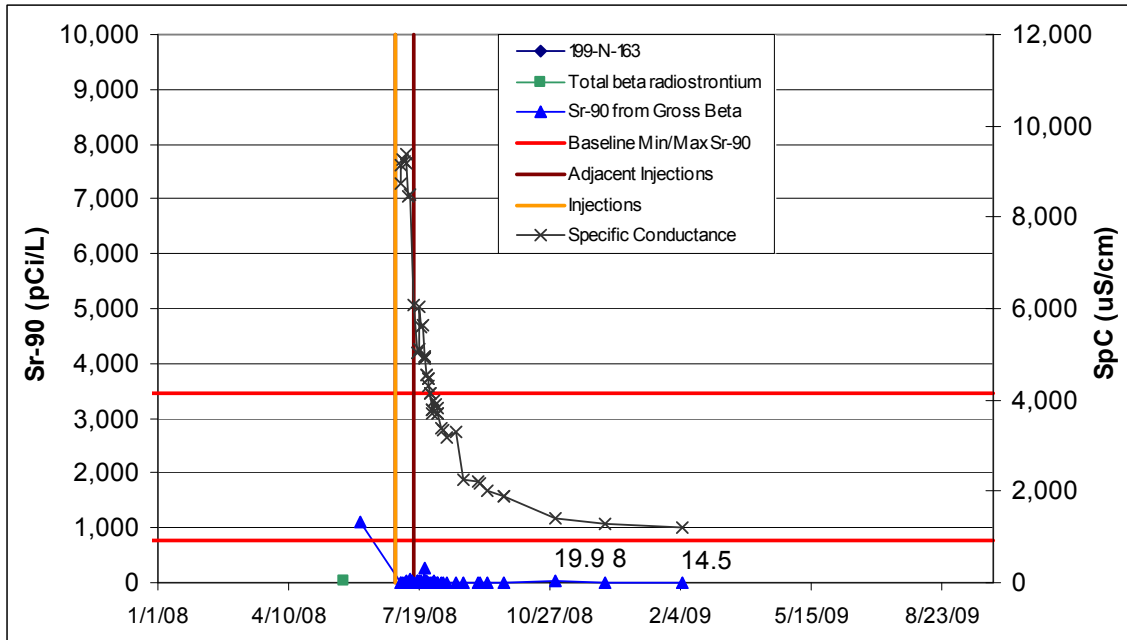
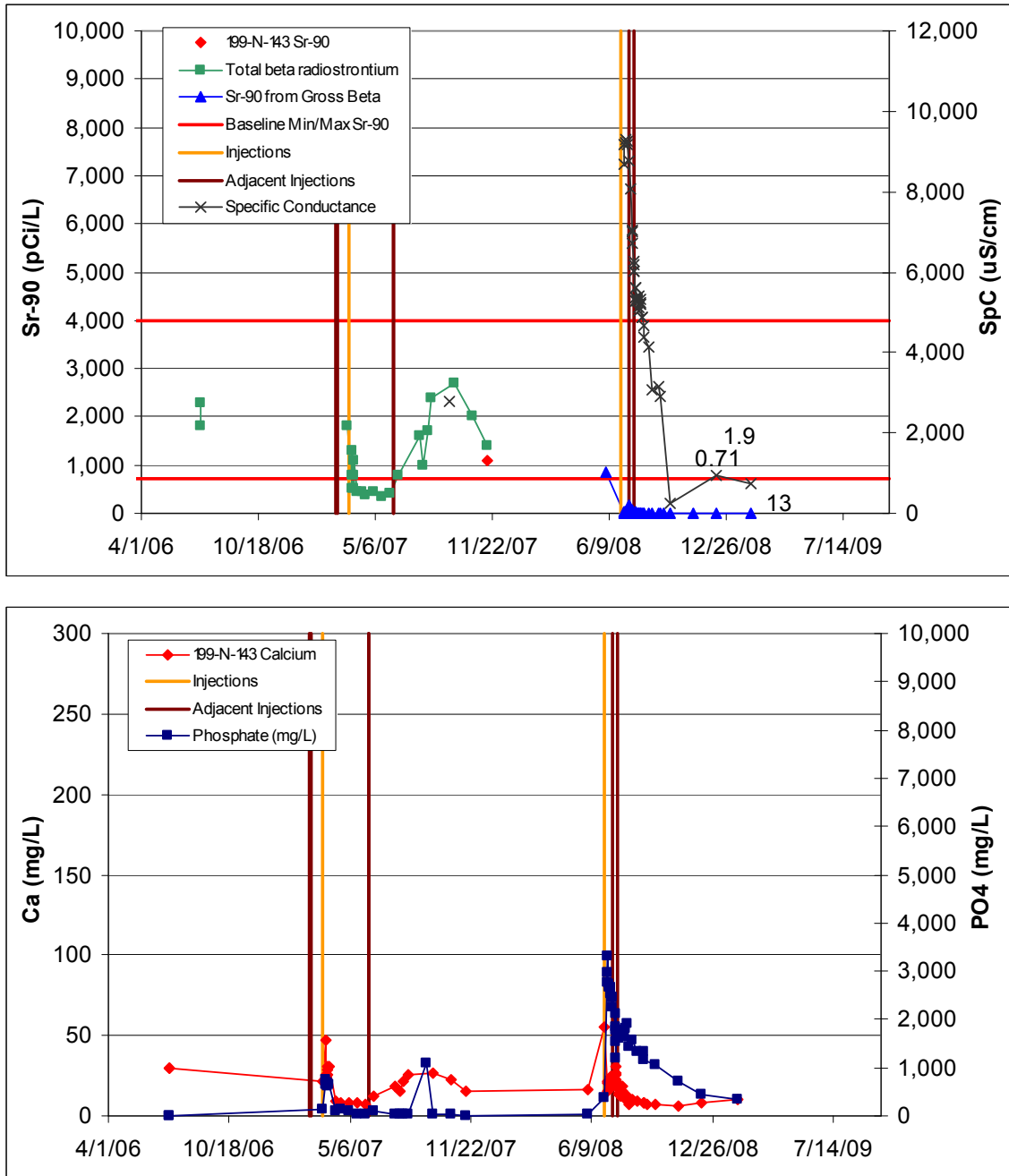


Figure B.13. Performance plots for well 199-N-163



**Figure B.14.** Performance plots for well 199-N-143

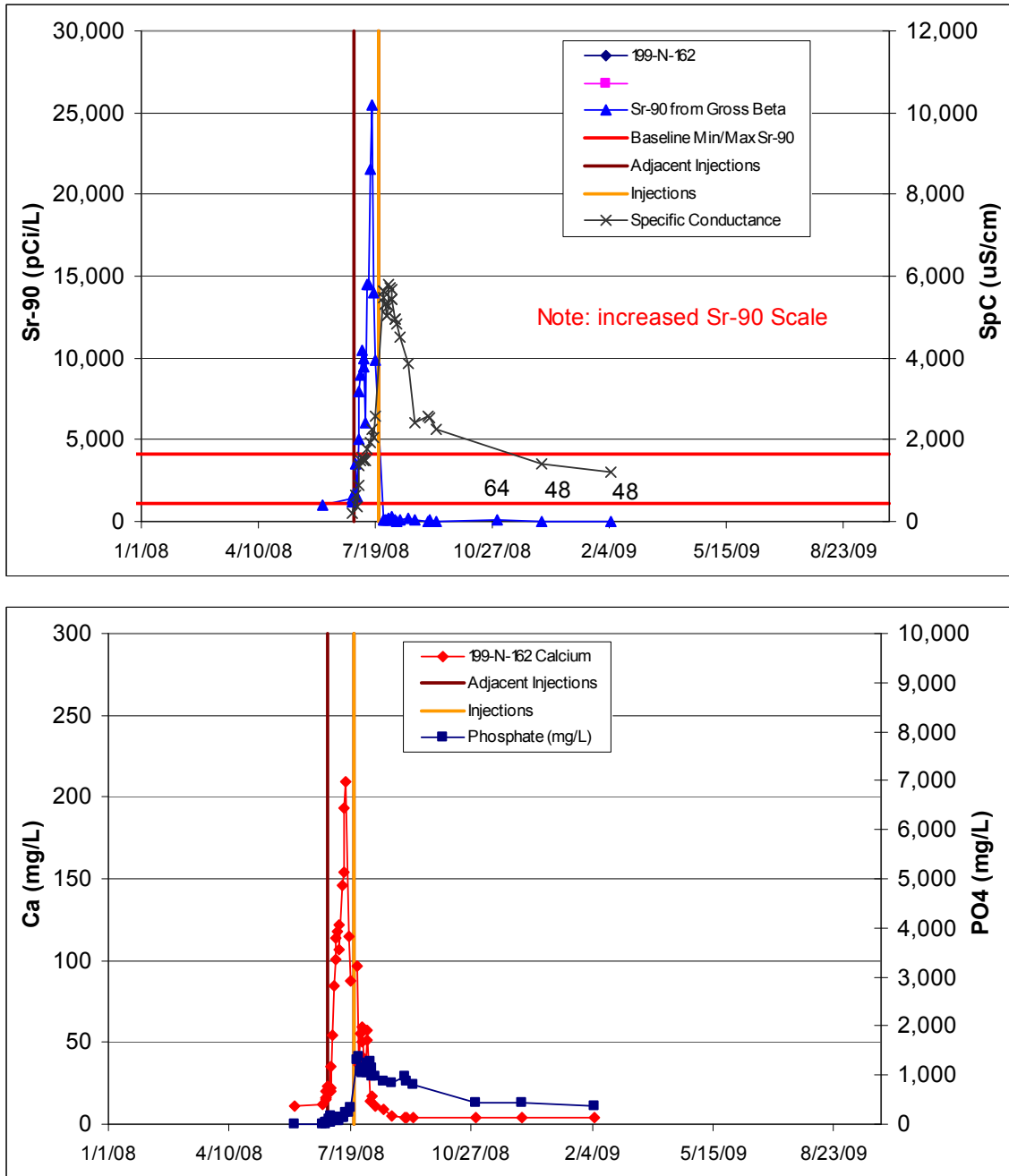
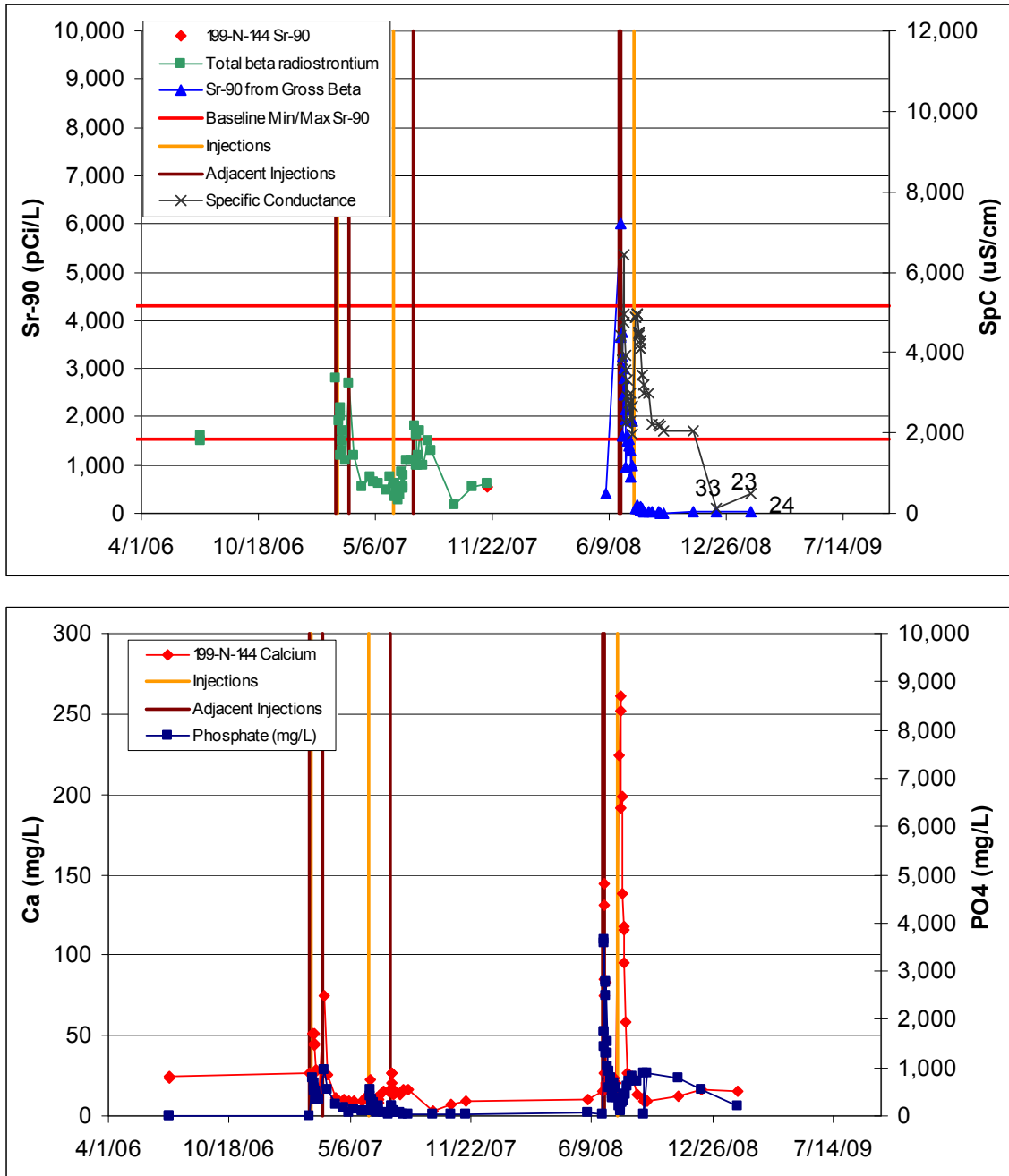


Figure B.15. Performance plots for well 199-N-162



**Figure B.16.** Performance plots for well 199-N-144

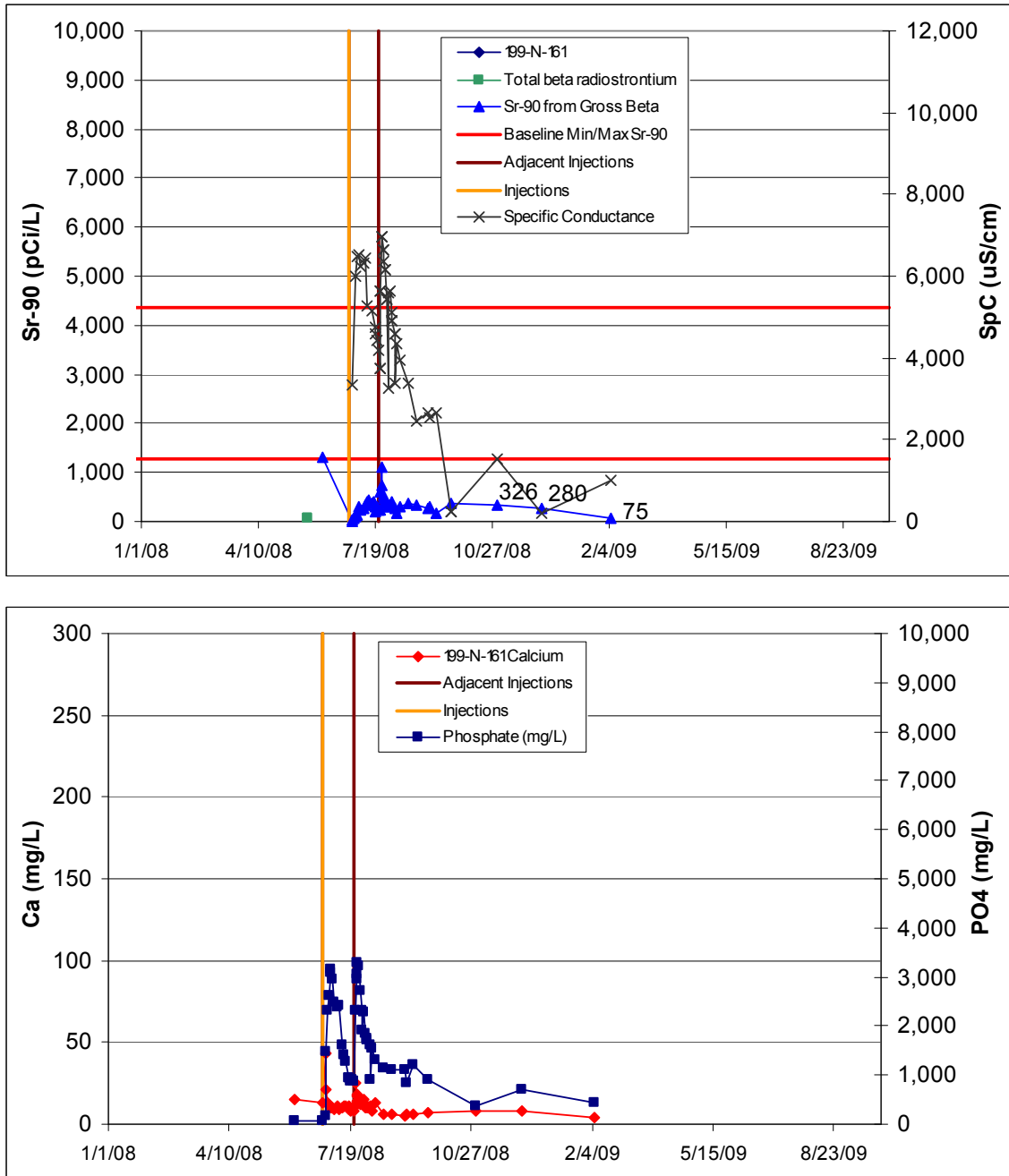
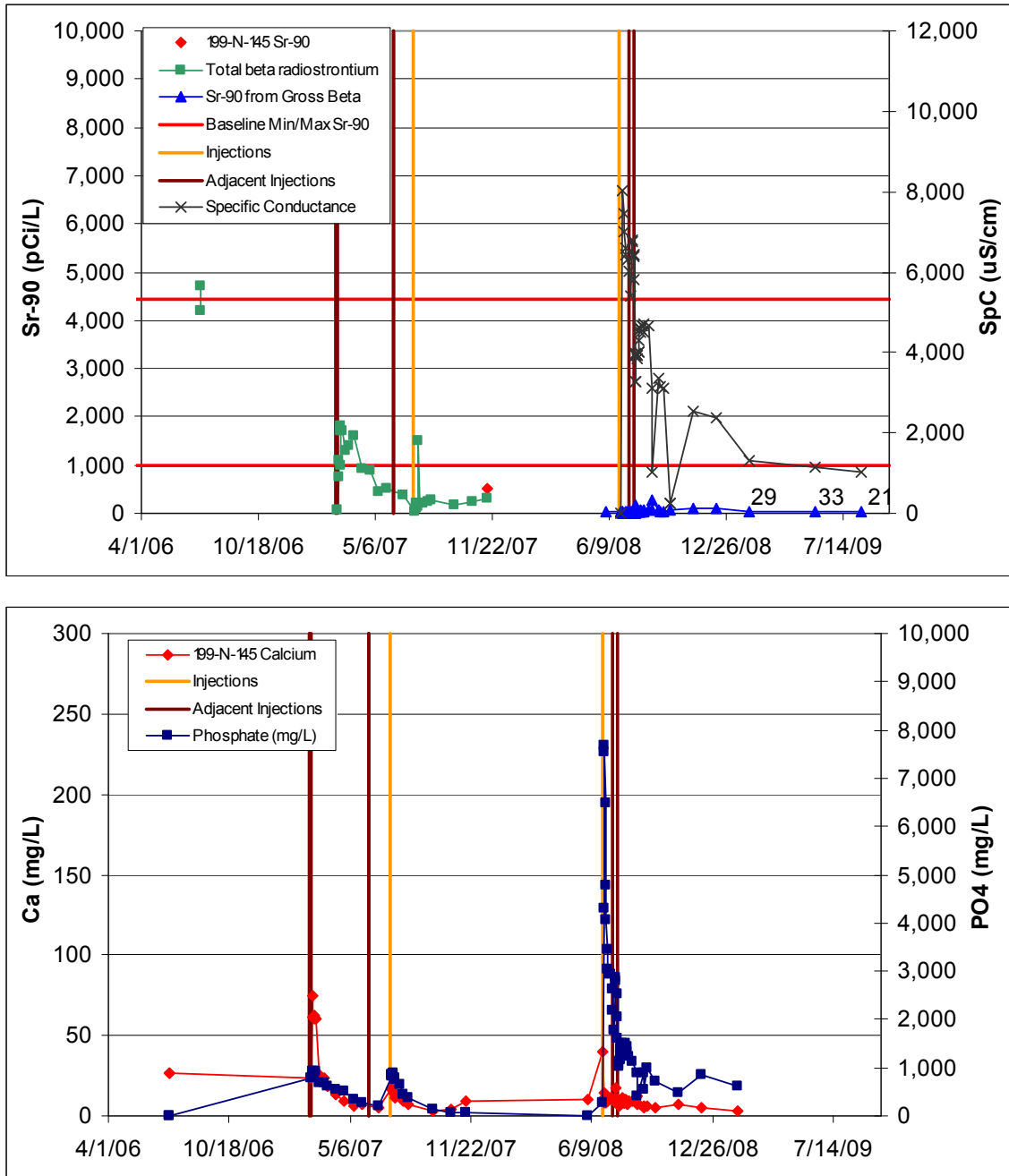


Figure B.17. Performance plots for well 199-N-161



**Figure B.18.** Performance plots for well 199-N-145



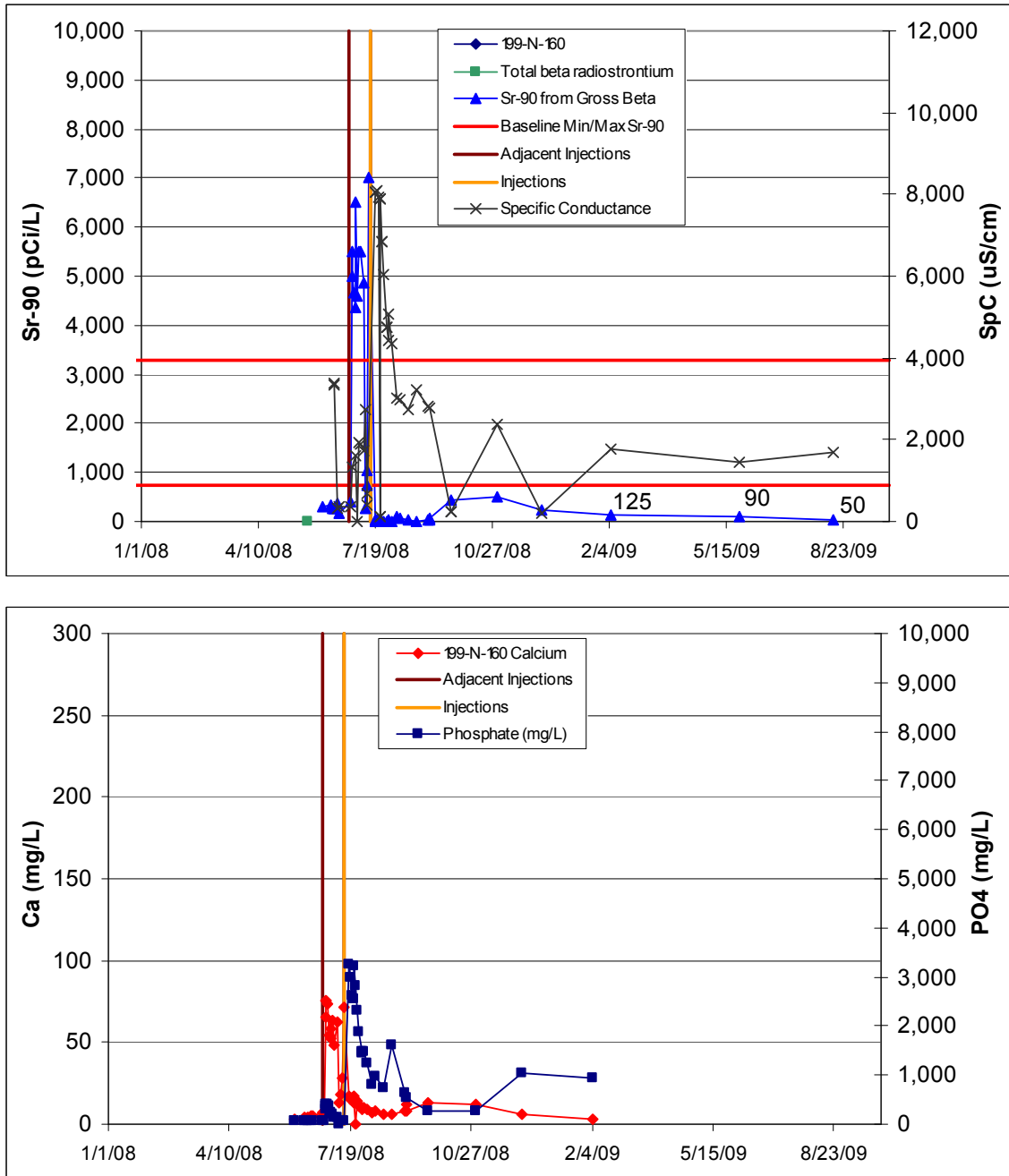


Figure B.19. Performance plots for well 199-N-160

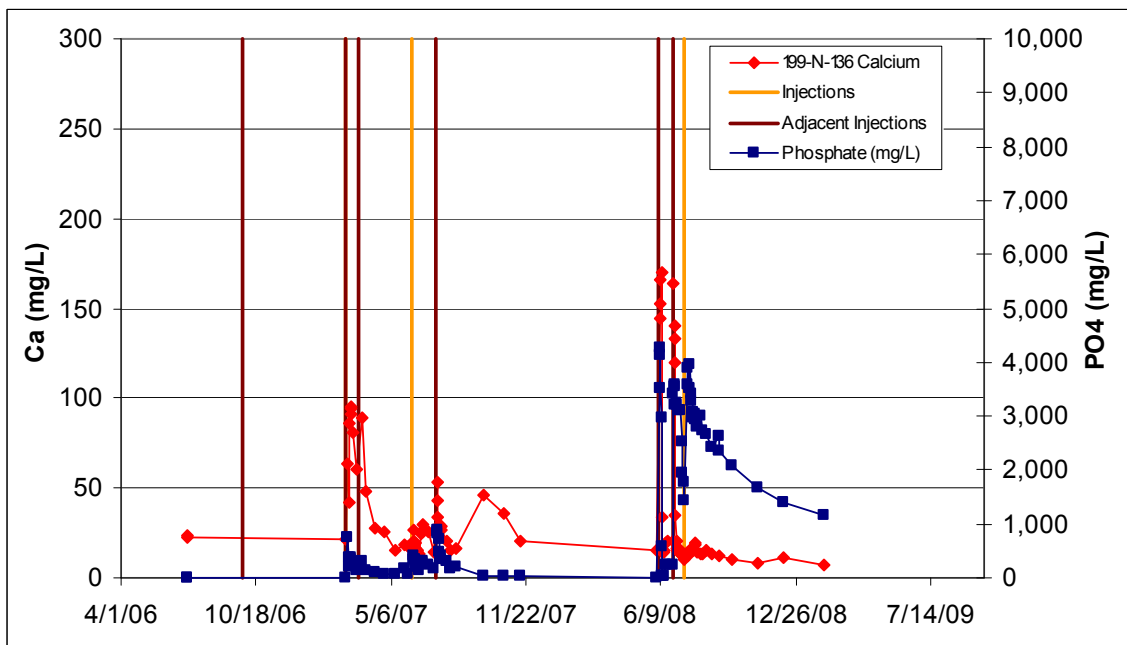
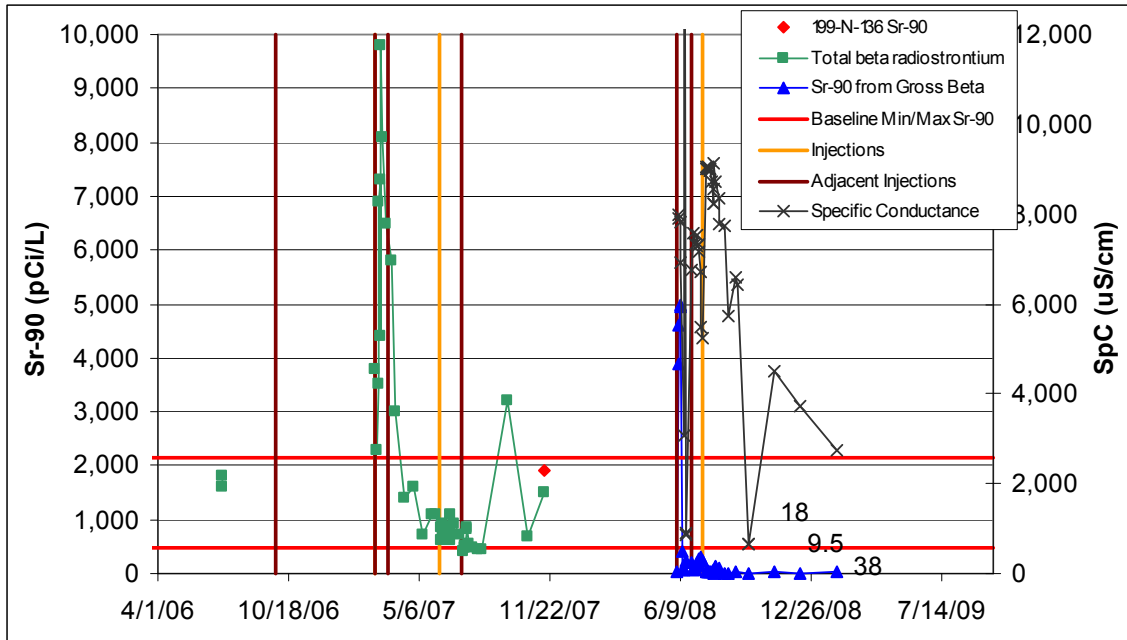


Figure B.20. Performance plots for well 199-N-136

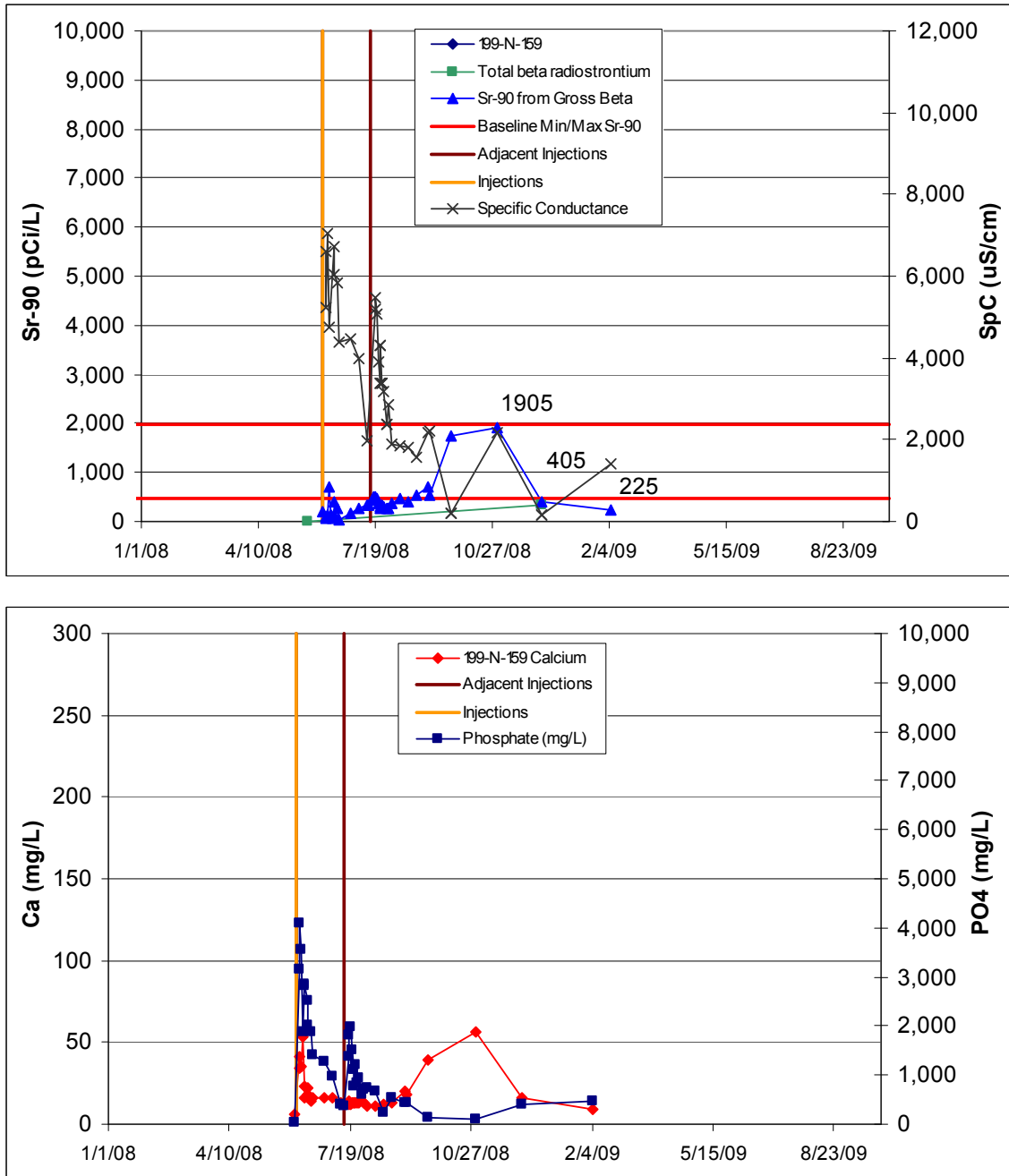


Figure B.21. Performance plots for well 199-N-159

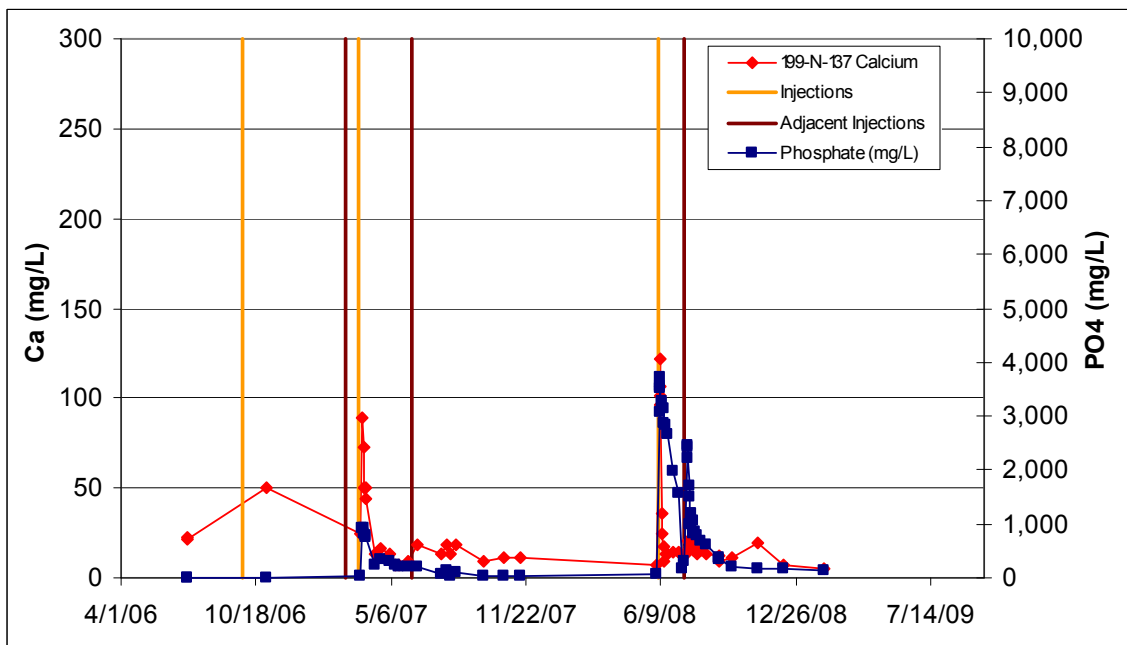
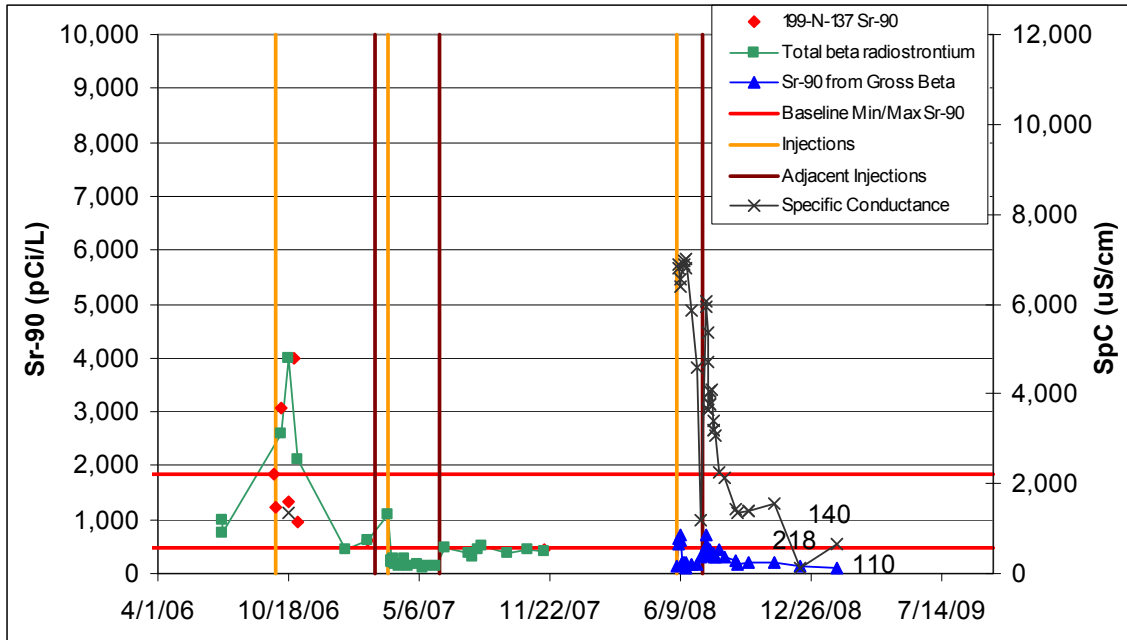


Figure B.22. Performance plots for well 199-N-137

## B.3 Pilot Test Site 1

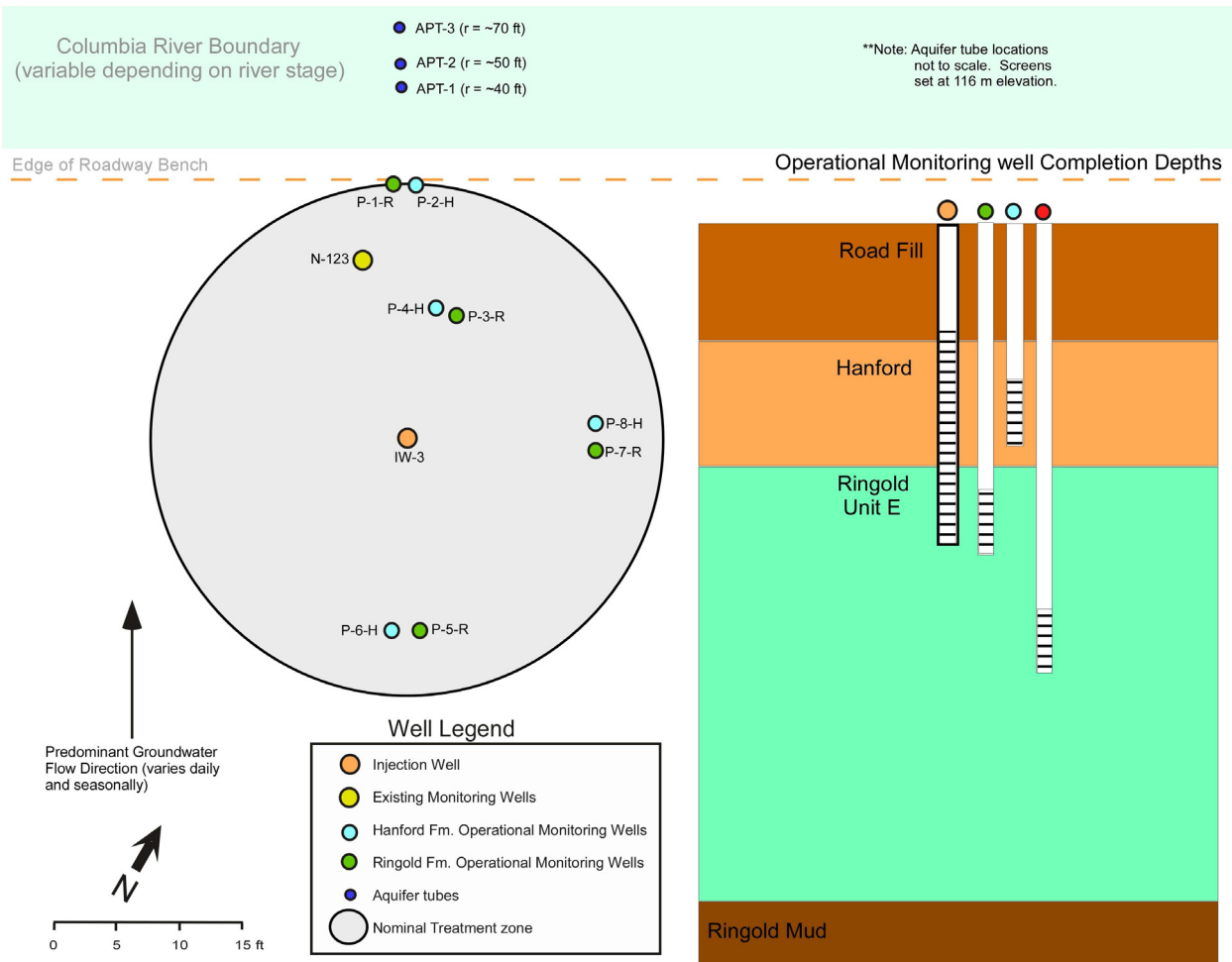
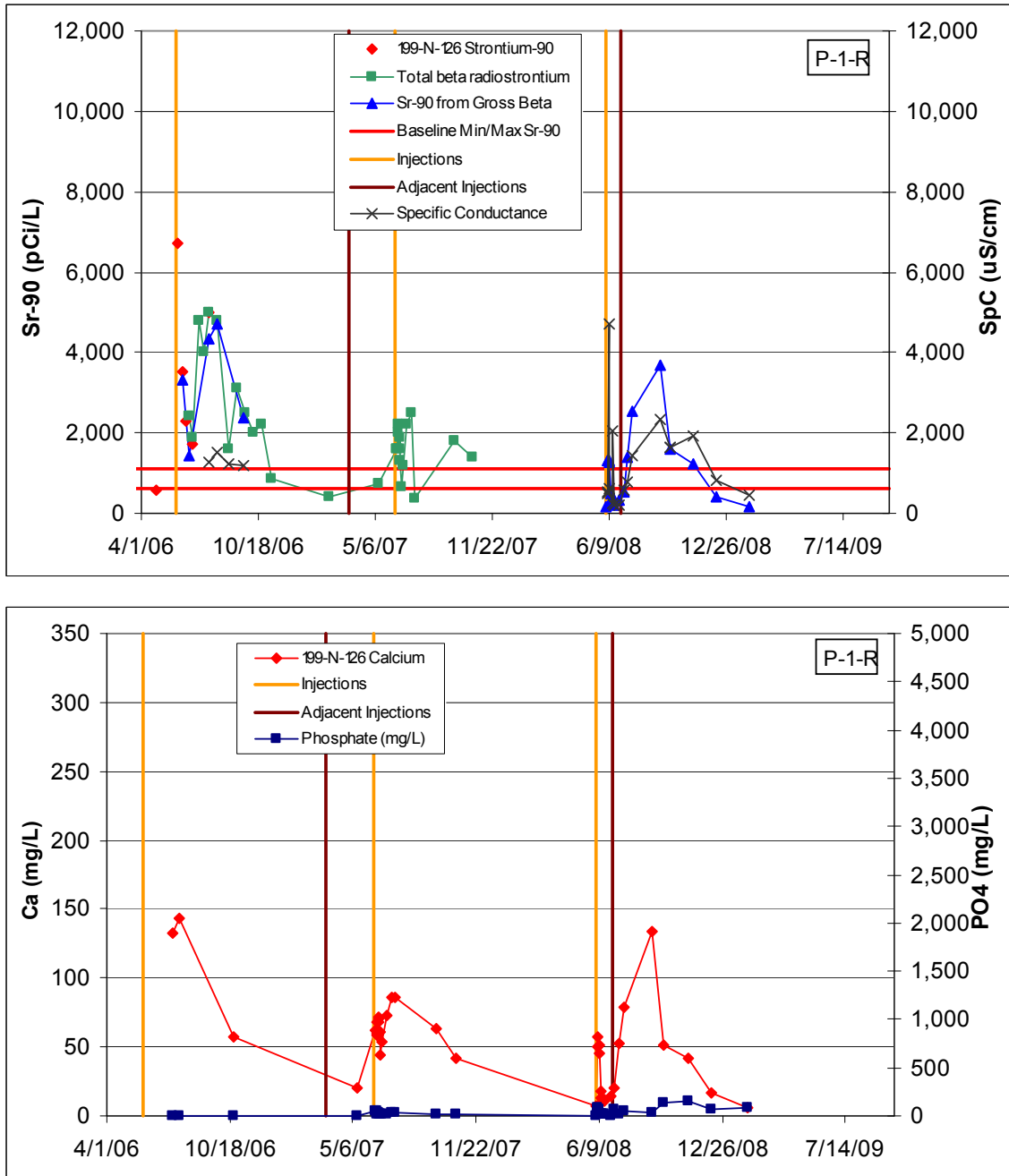
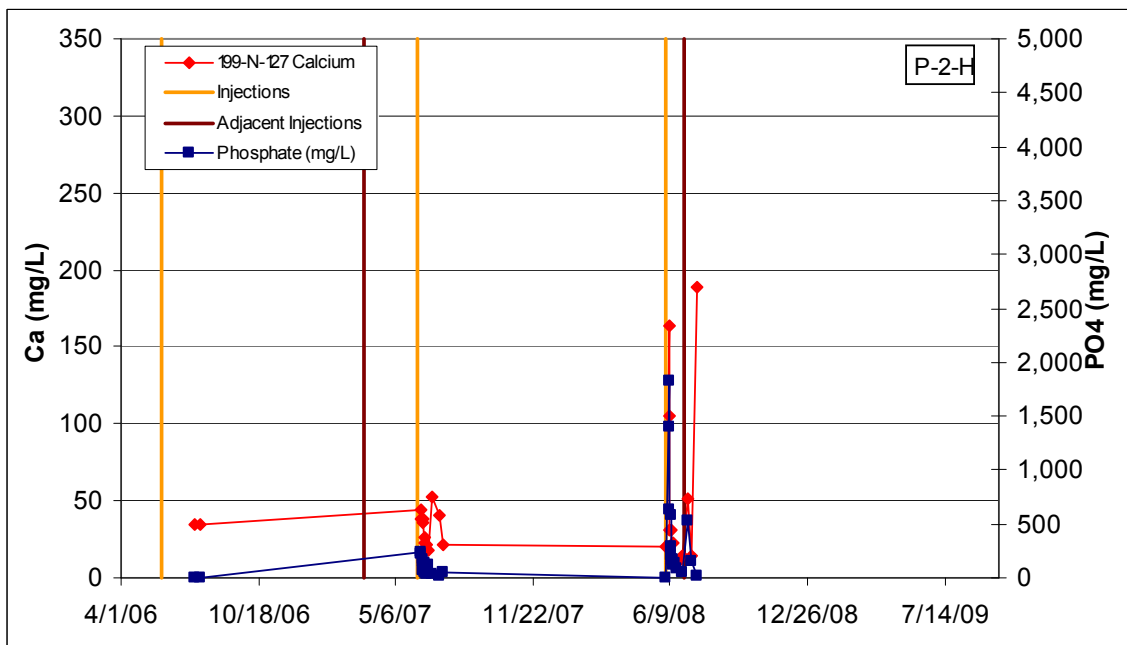
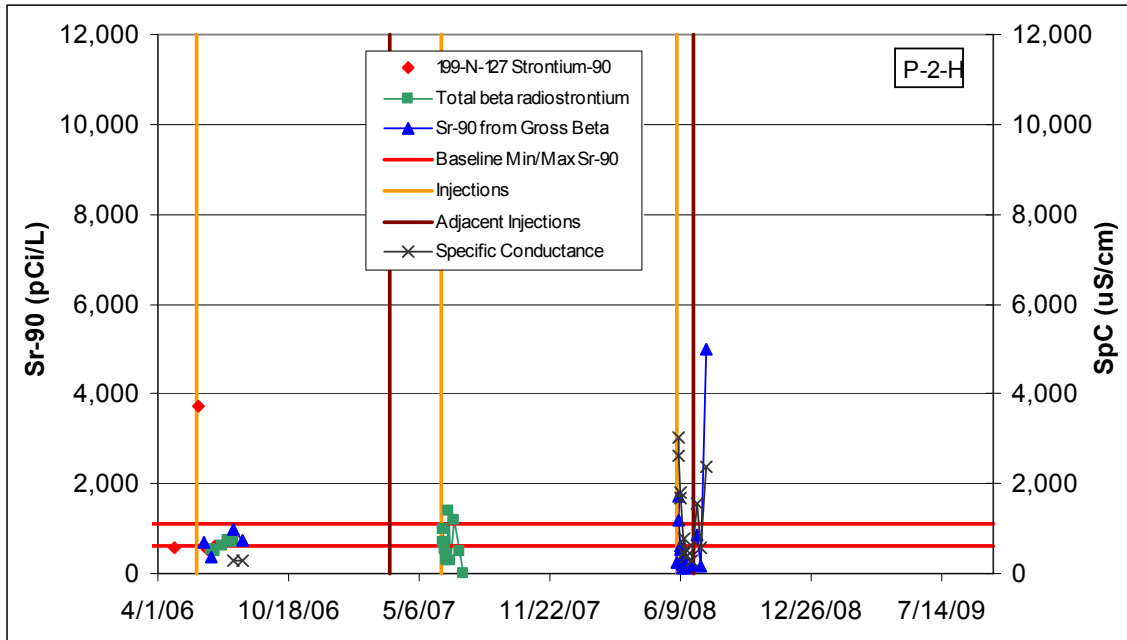


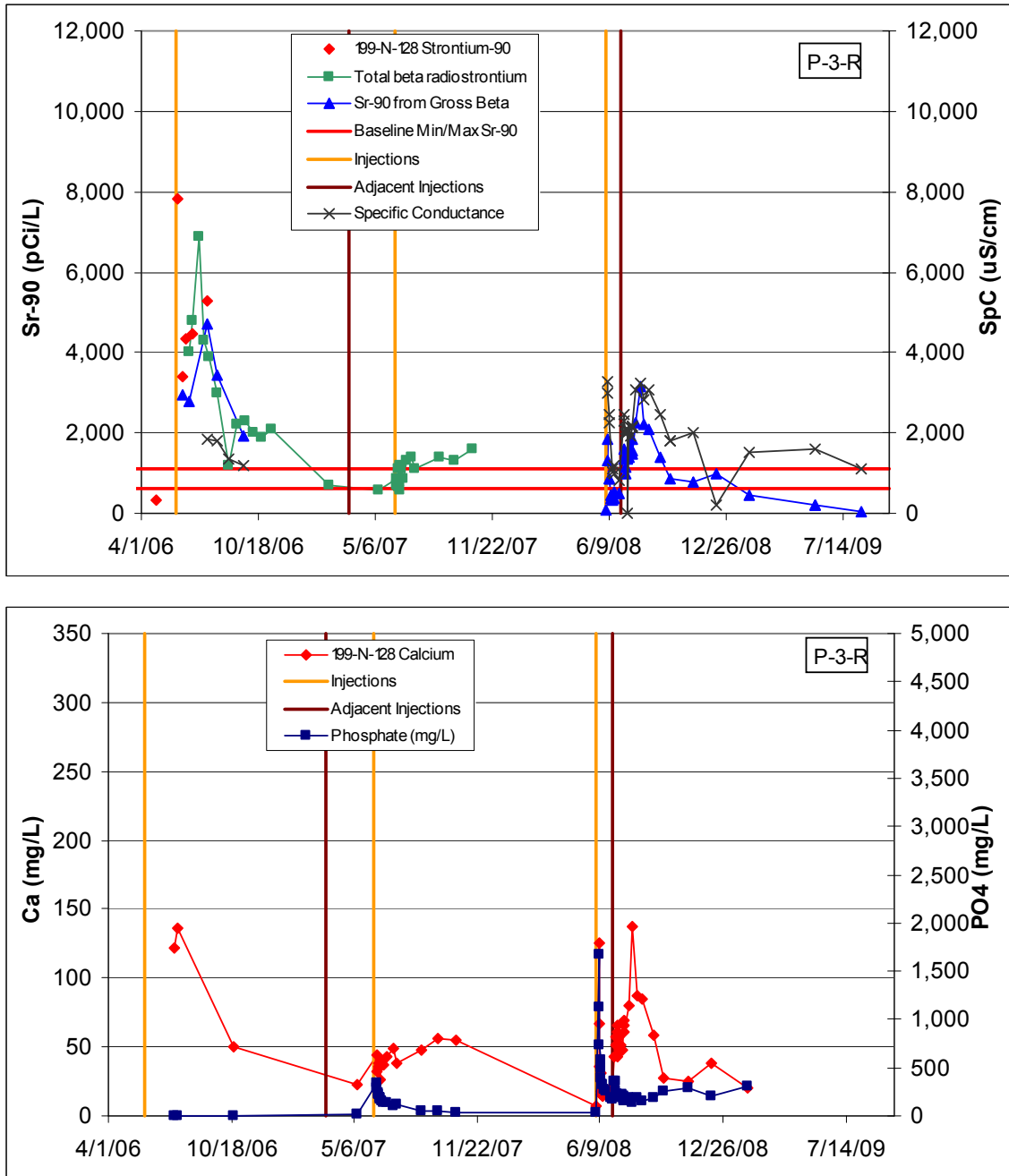
Figure B.23. Map of pilot test site 1



**Figure B.24.** Performance plots for well 199-N-126 (P-1-R)

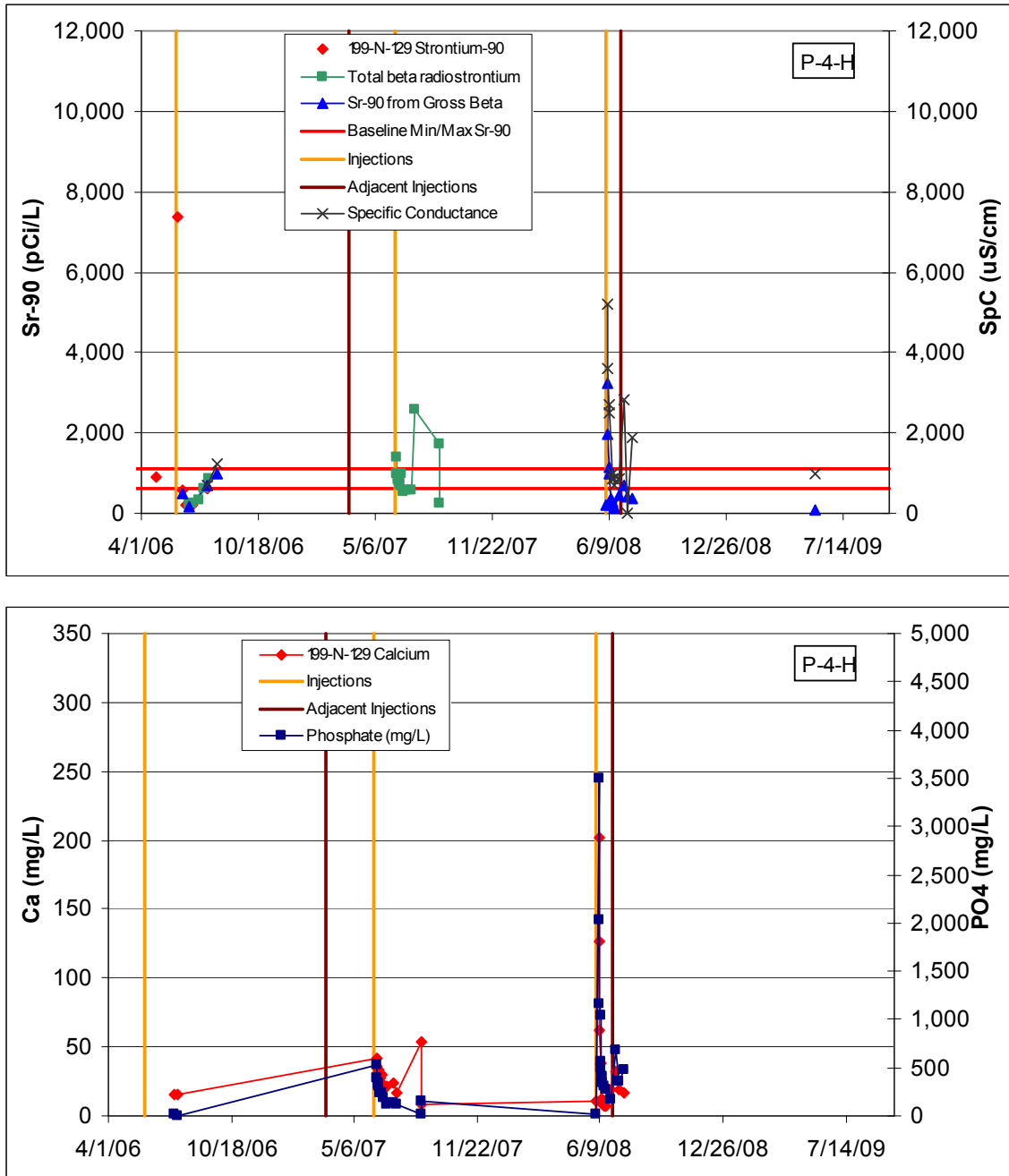


**Figure B.25.** Performance plots for well 199-N-127 (P-2-H)

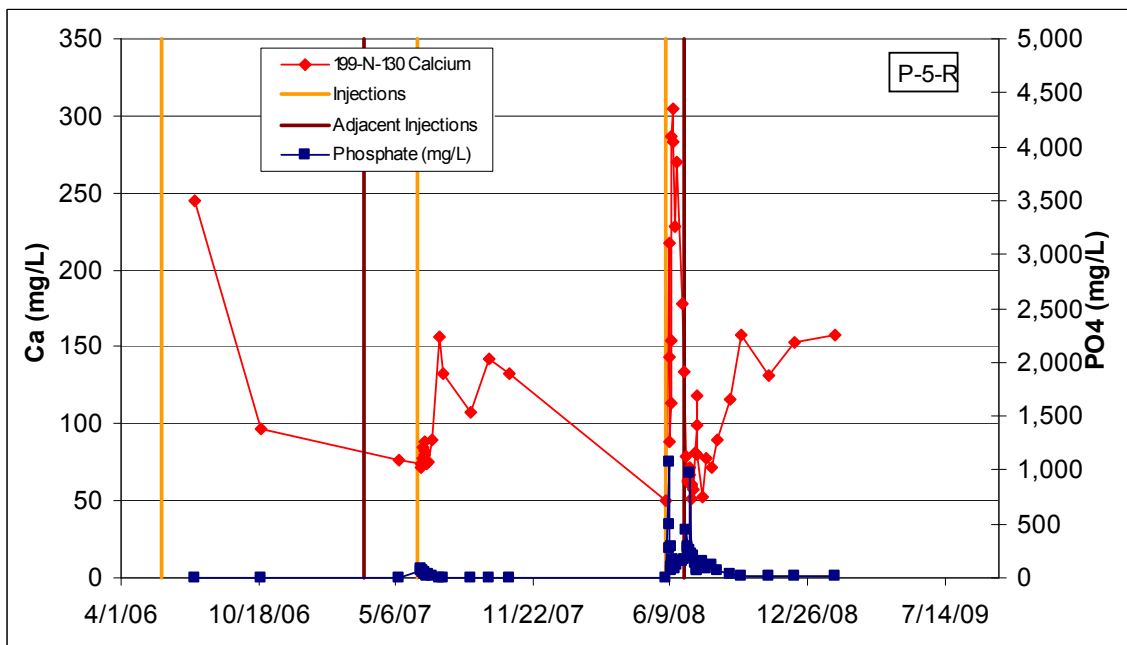
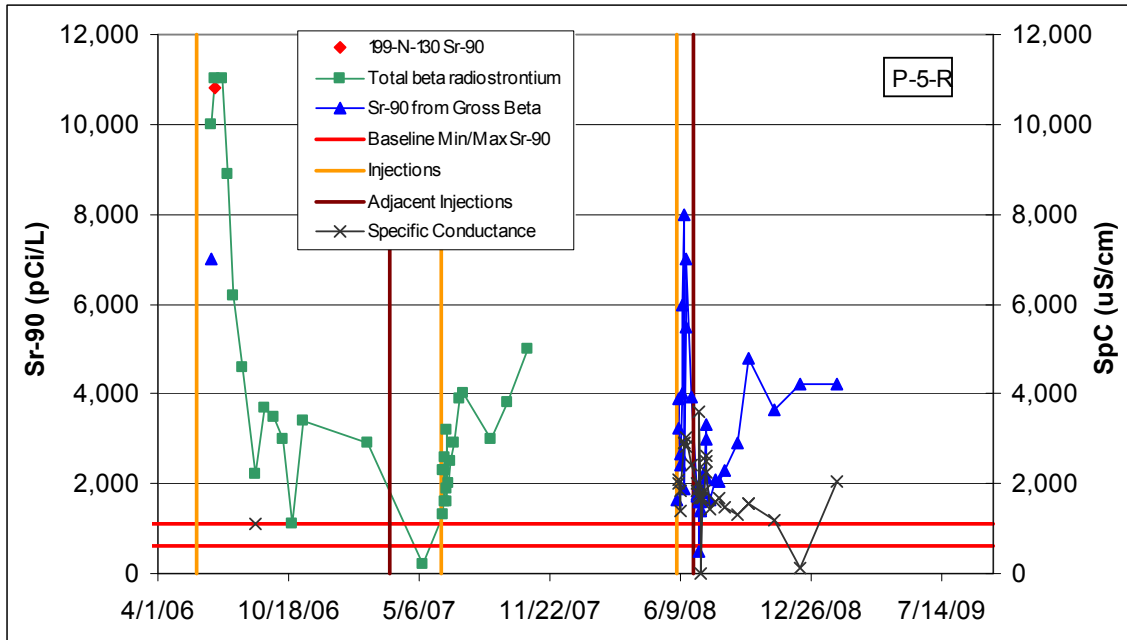


**Figure B.26.** Performance plots for well 199-N-128 (P-3-R)

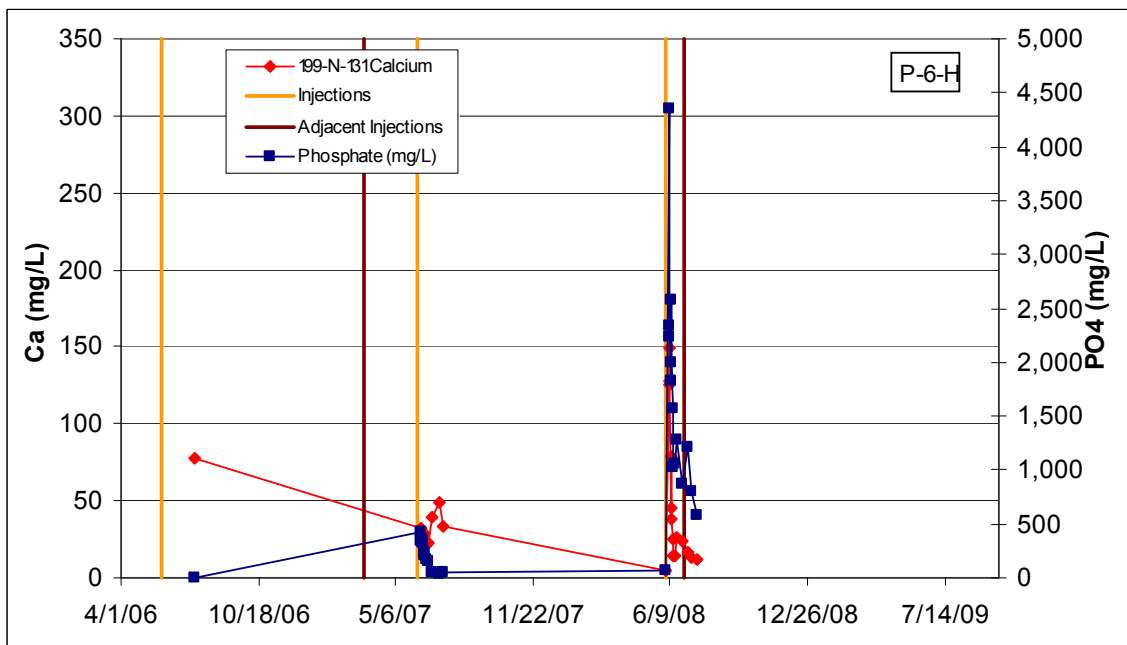
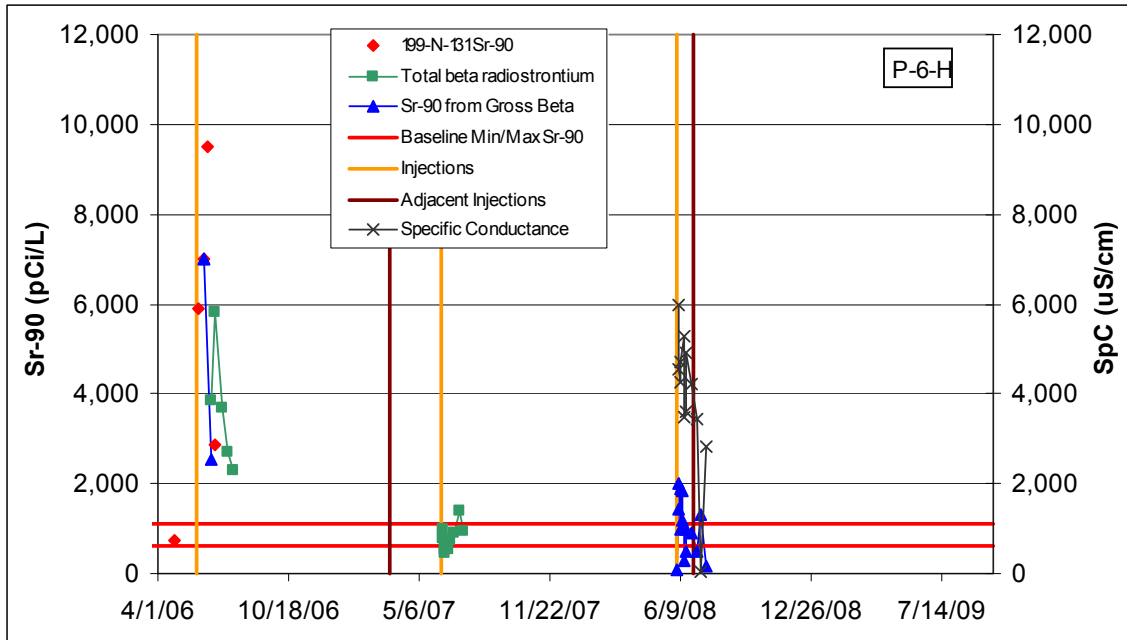




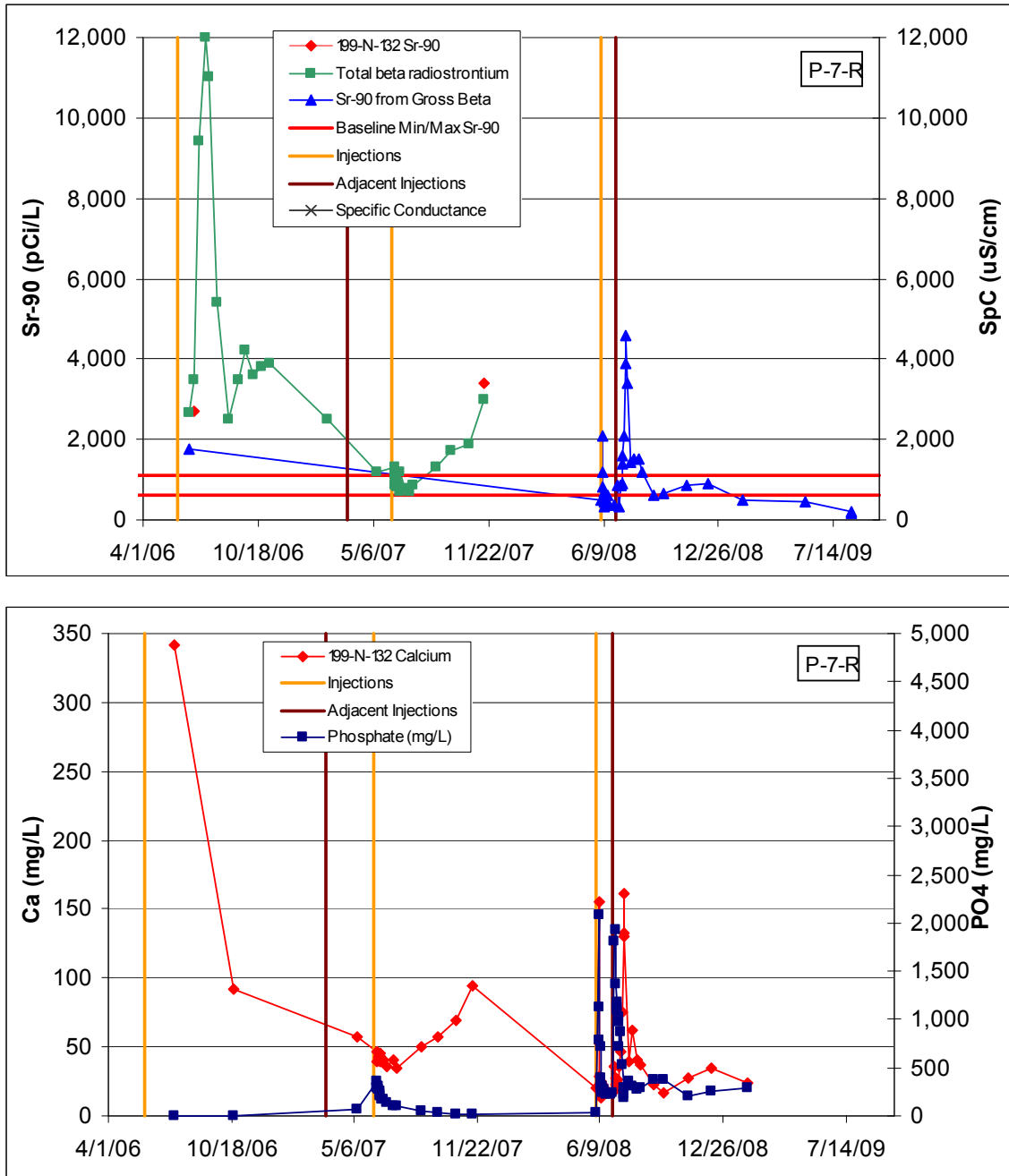
**Figure B.27.** Performance plots for well 199-N-129 (P-4-H)



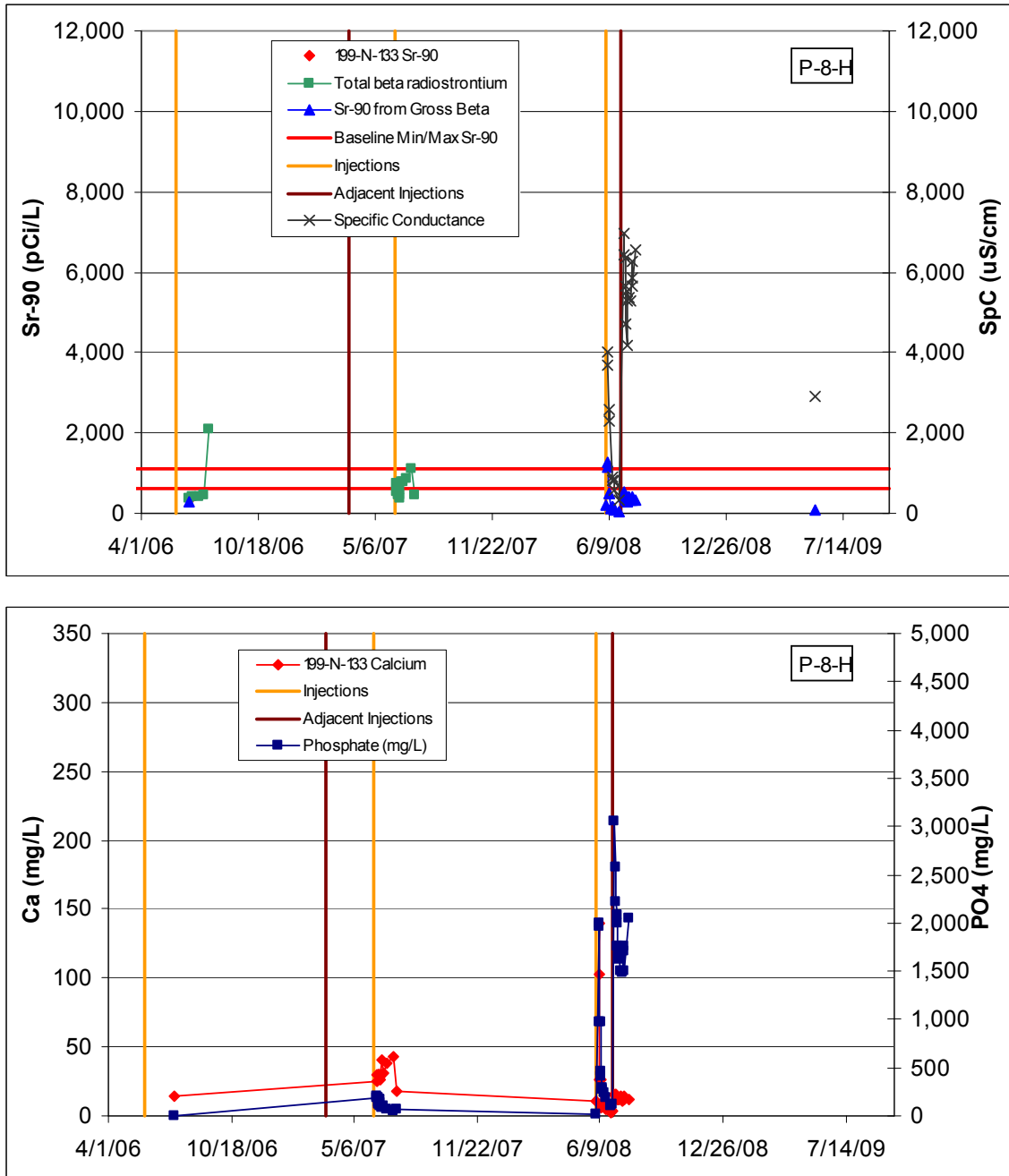
**Figure B.28.** Performance plots for well 199-N-130 (P-5-R)



**Figure B.29.** Performance plots for well 199-N-131 (P-6-H)



**Figure B.30.** Performance plots for well 199-N-132 (P-7-R)



**Figure B.31.** Performance plots for well 199-N-133 (P-8-H)

## B.4 Pilot Test Site 2

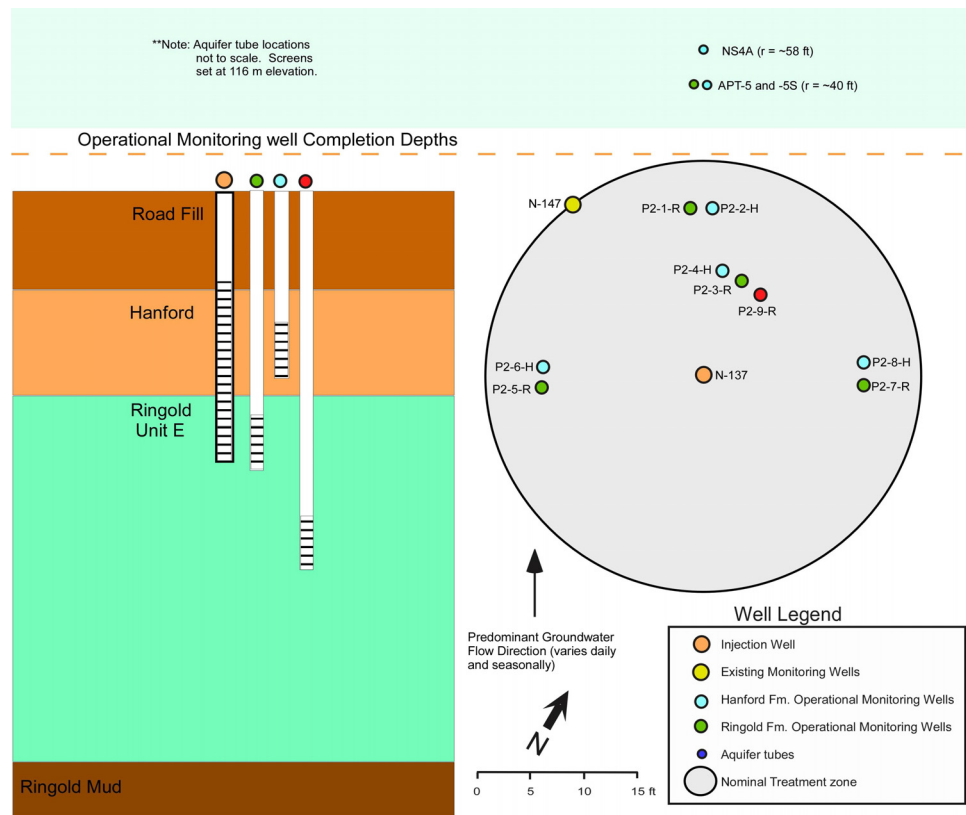
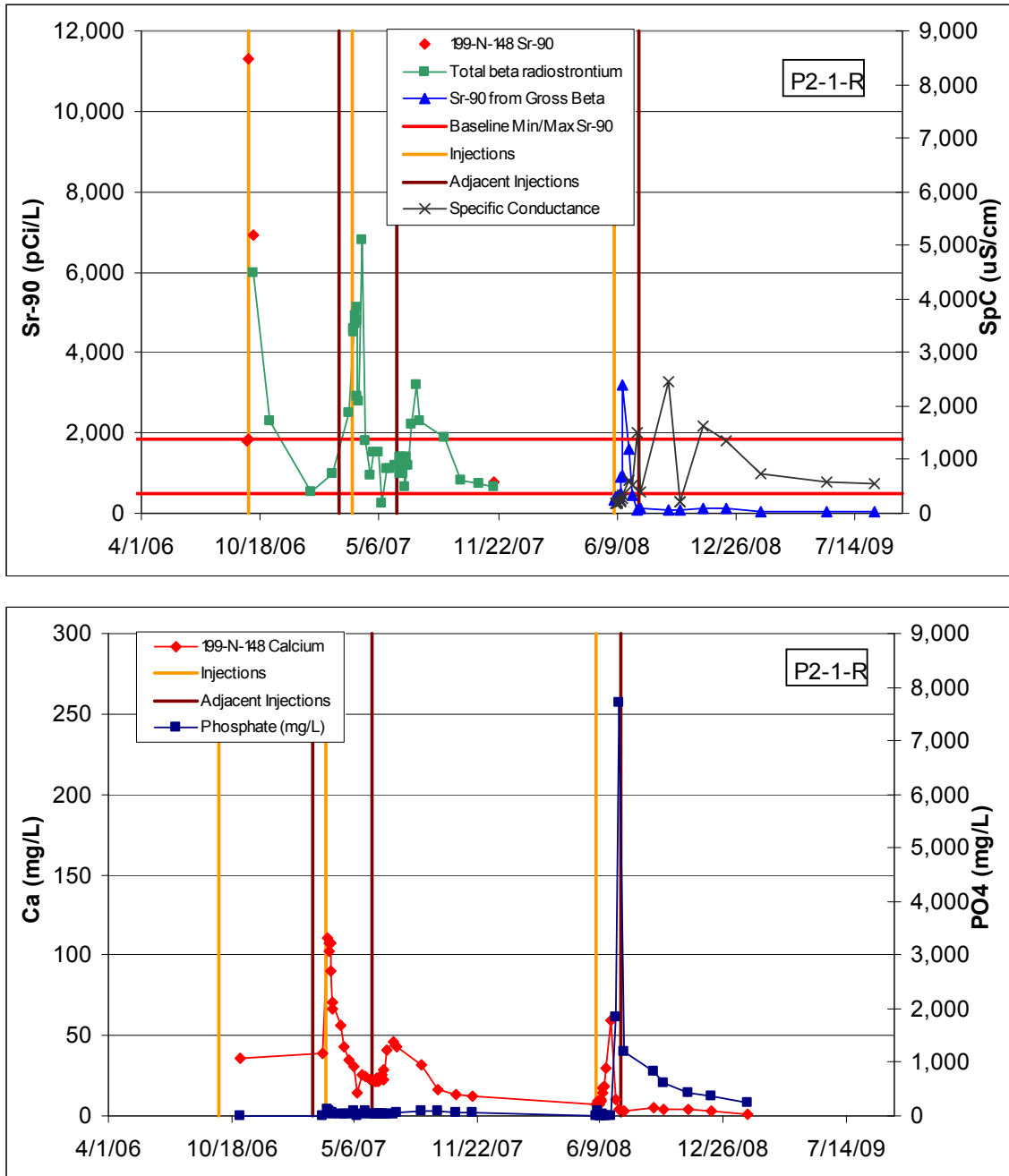
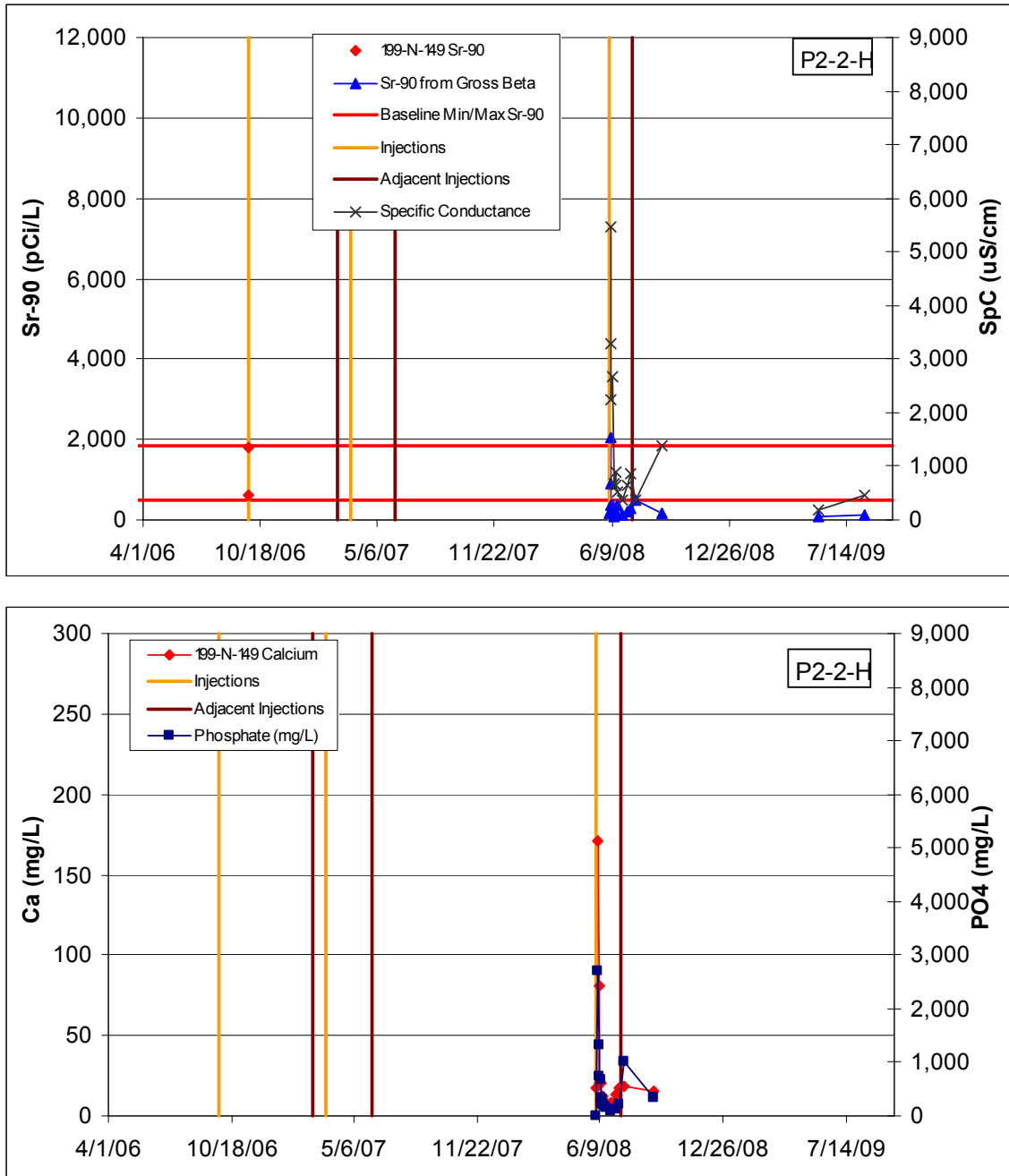


Figure B.32. Pilot test site 2

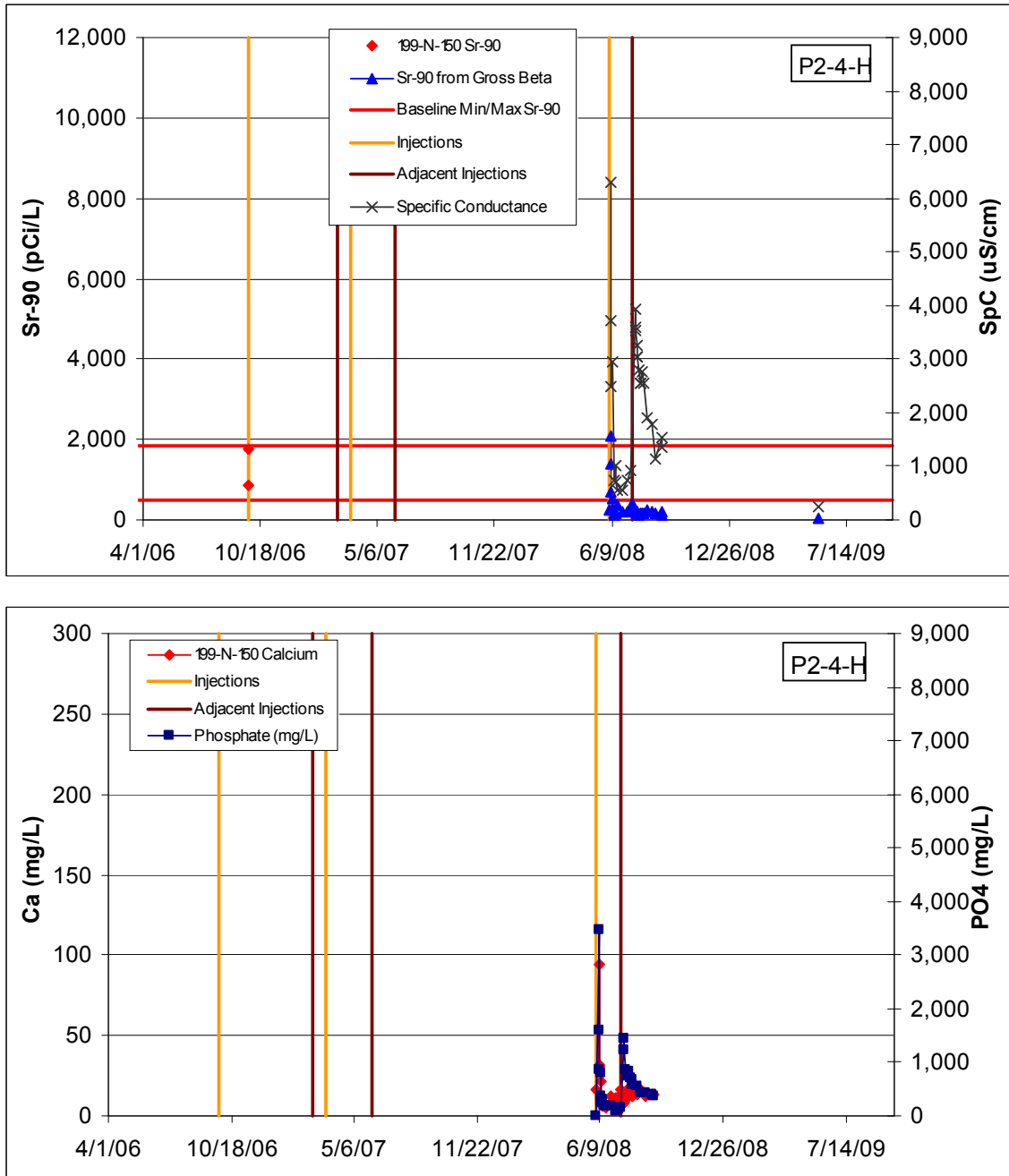


**Figure B.33.** Performance plots for well 199-N-148 (P2-1-R)

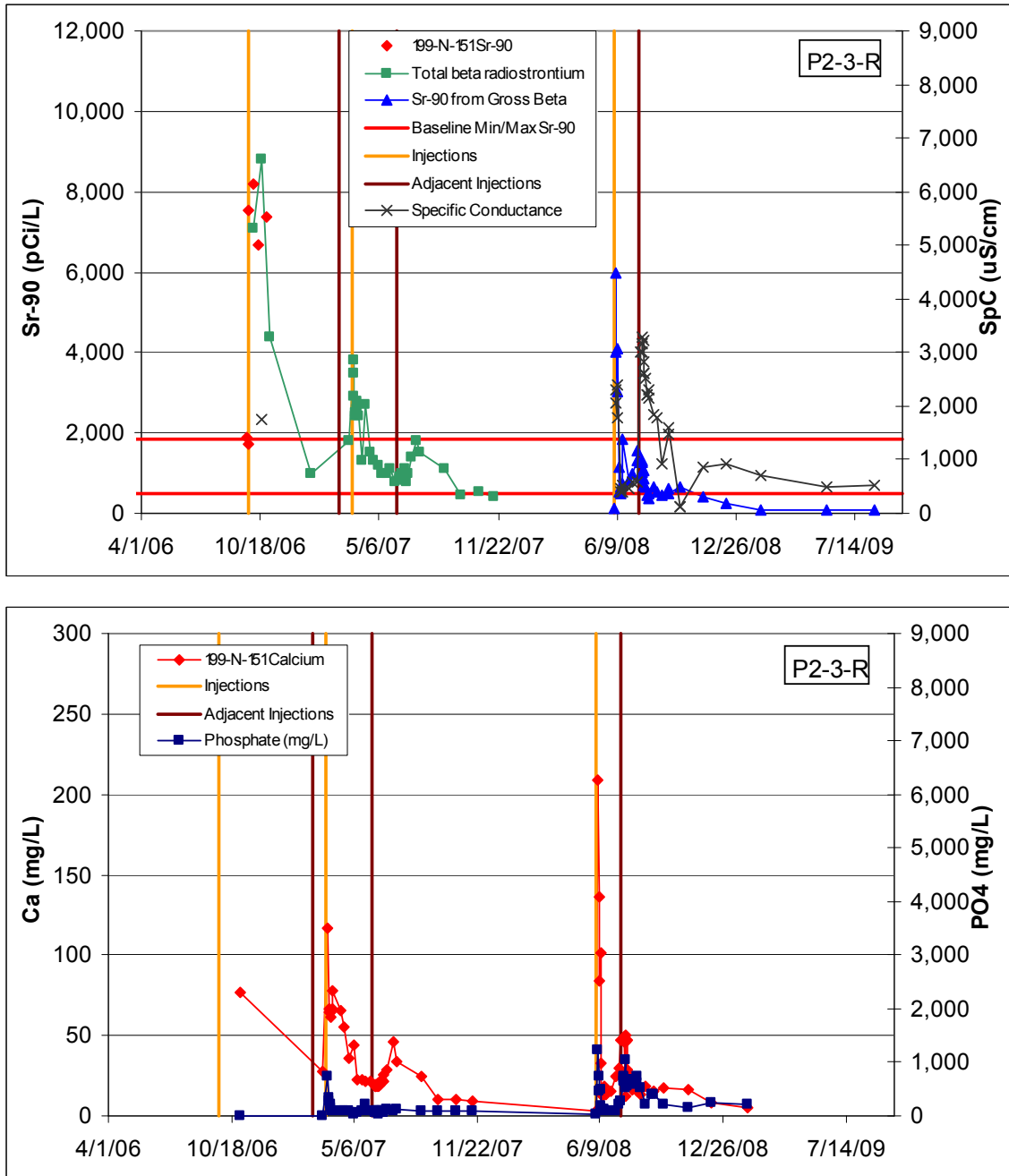


**Figure B.34.** Performance plots for well 199-N-149 (P2-2-H)

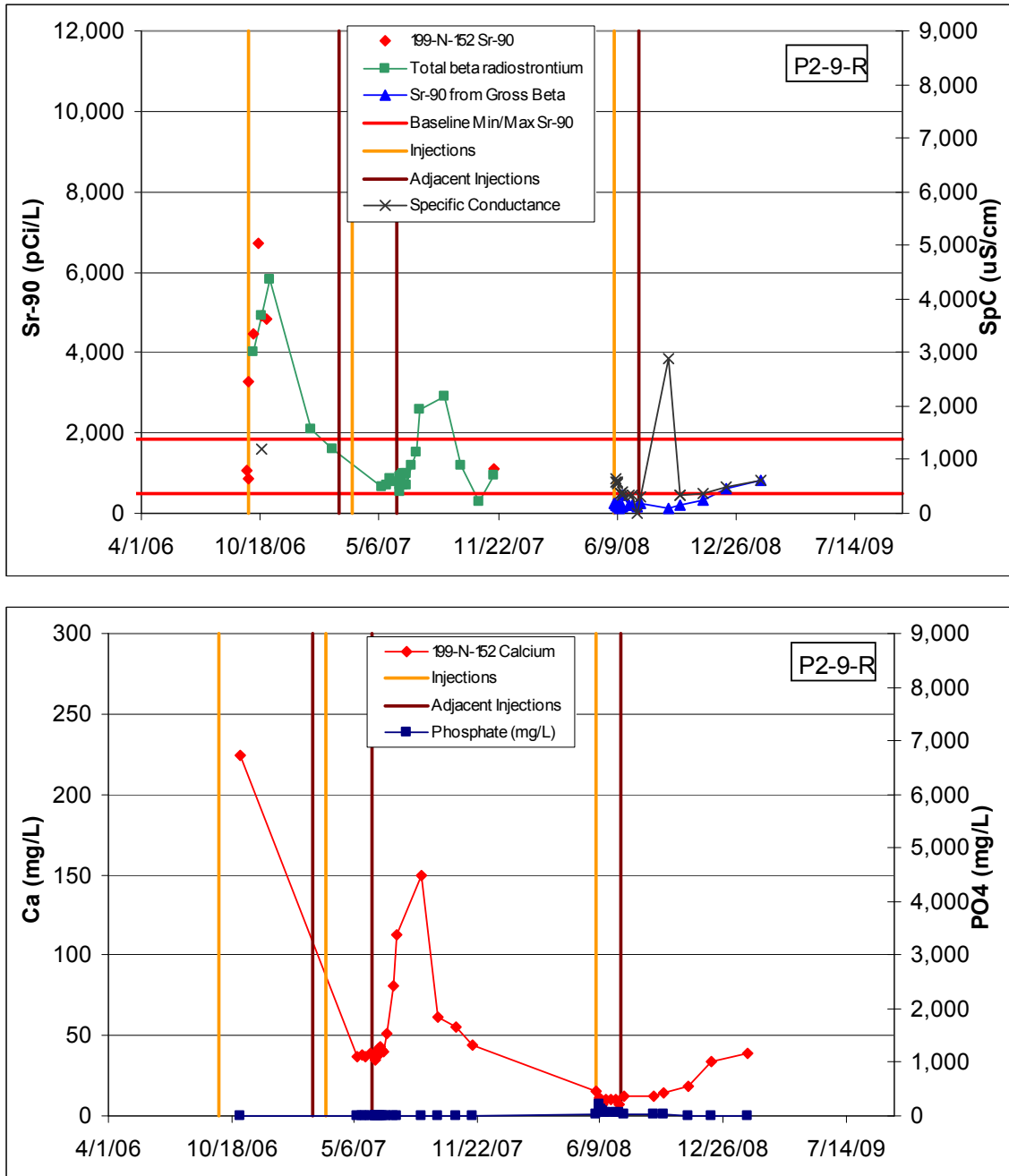




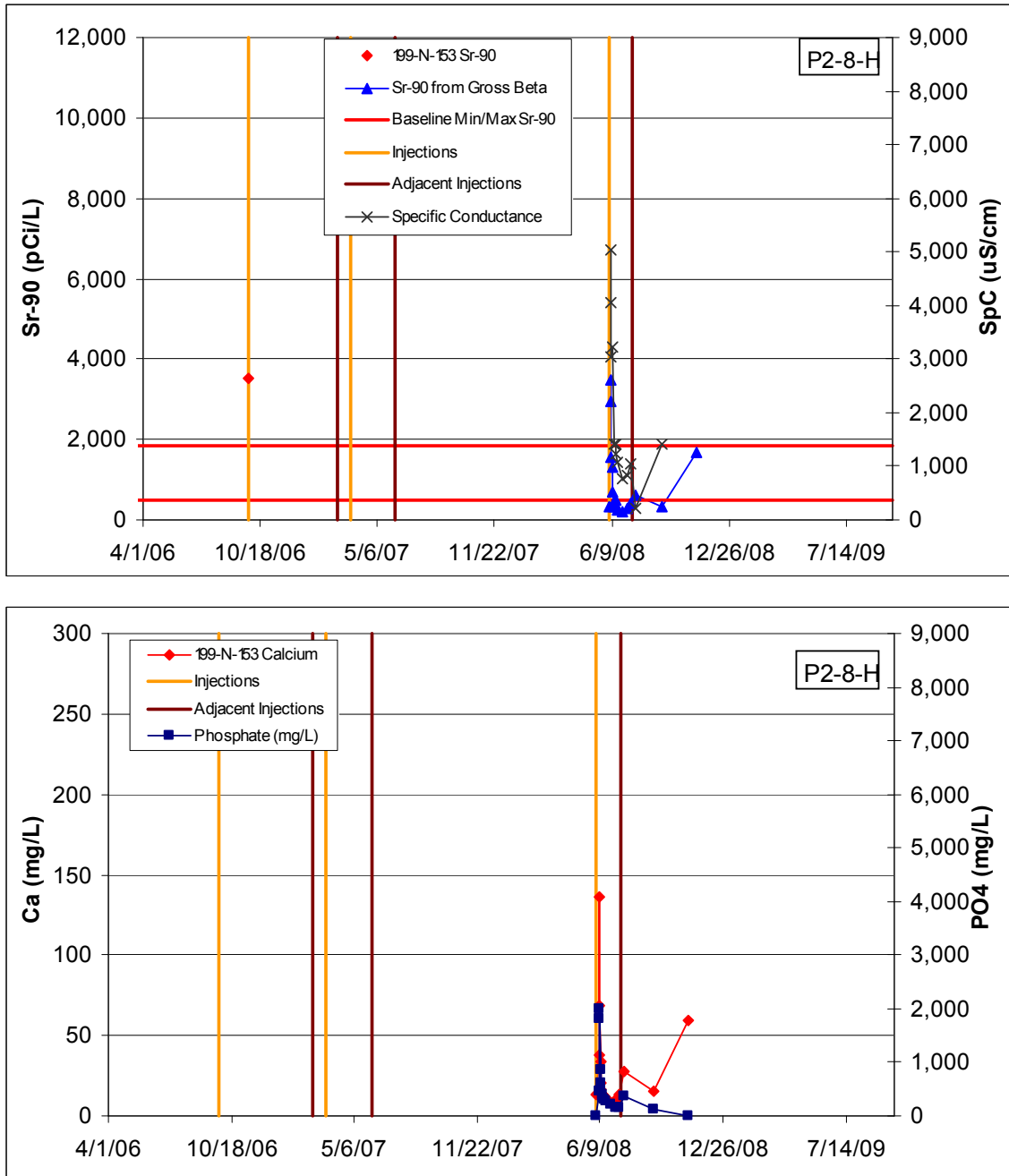
**Figure B.35.** Performance plots for well 199-N-150 (P2-4-H)



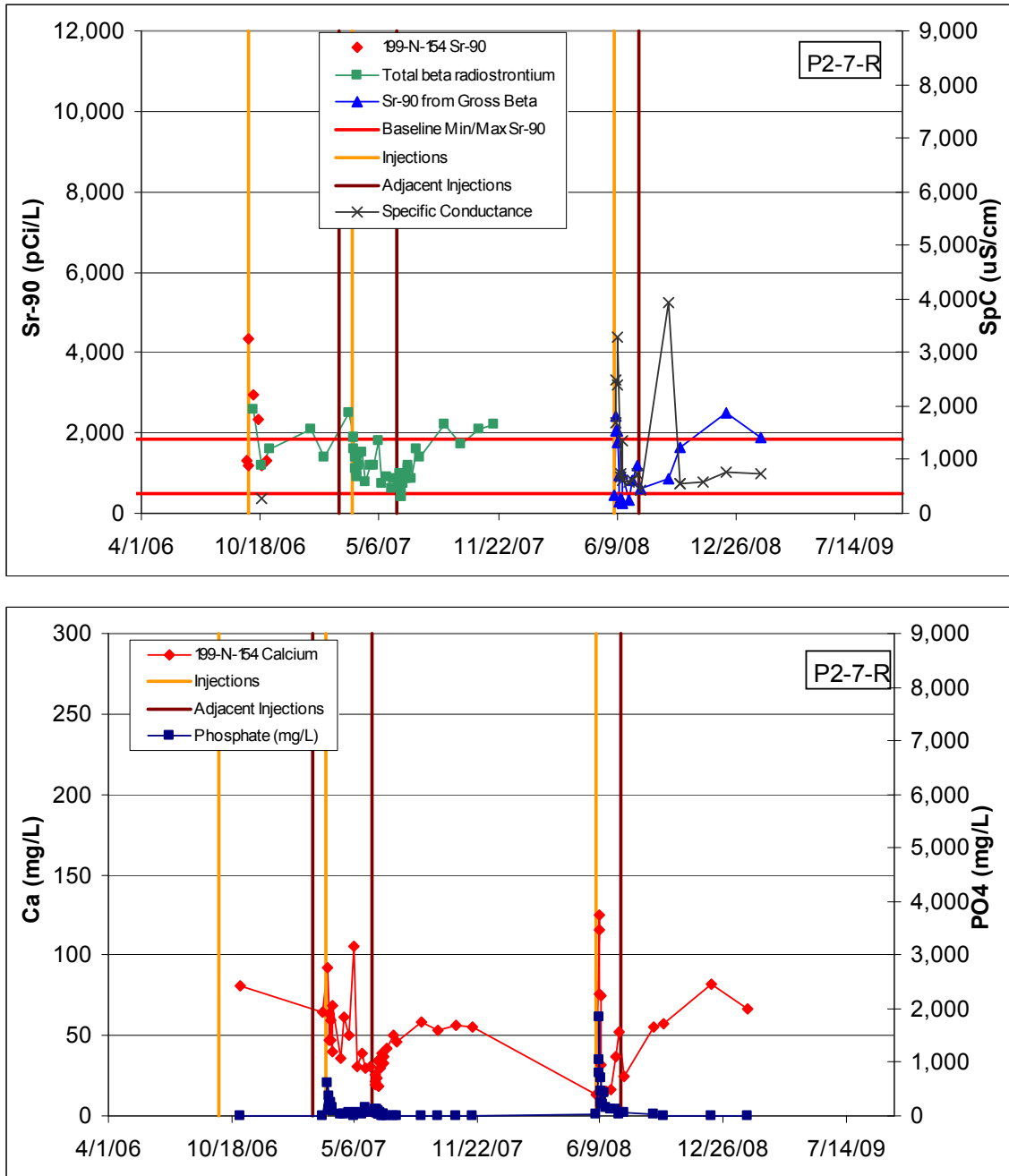
**Figure B.36.** Performance plots for well 199-N-151 (P2-3-R)



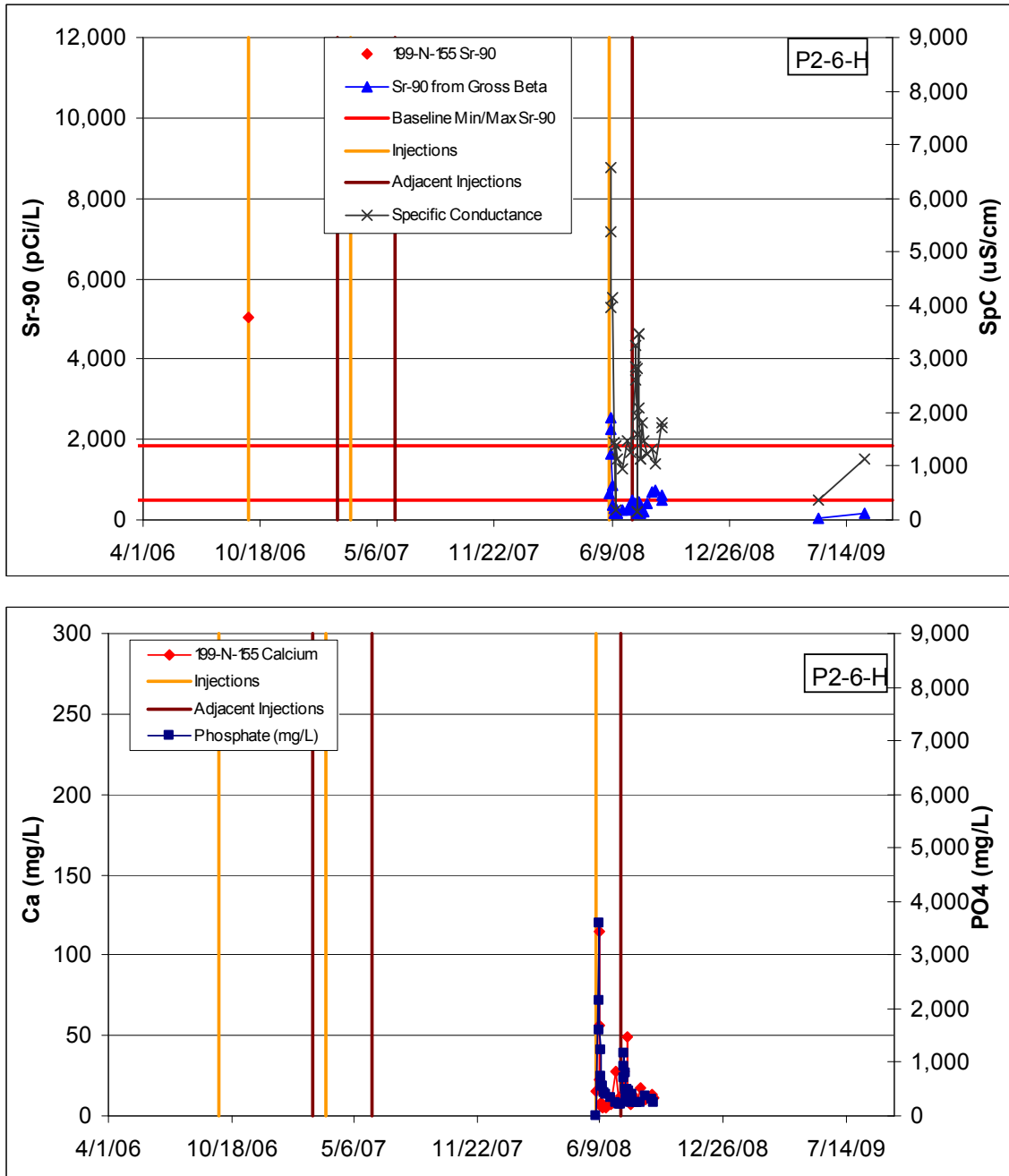
**Figure B.37.** Performance plots for well 199-N-152 (P2-9-R)



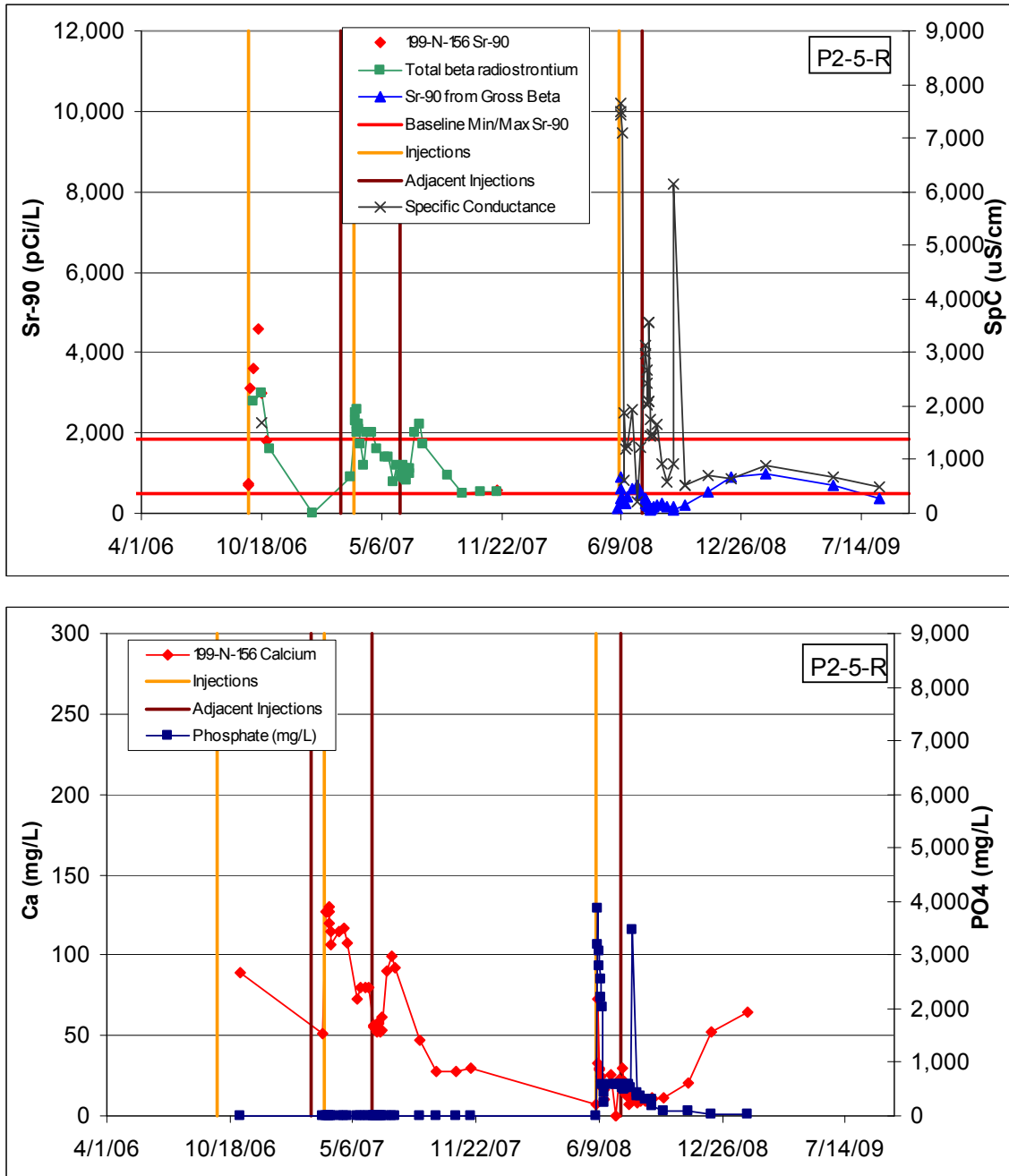
**Figure B.38.** Performance plots for well 199-N-153 (P2-8-H)



**Figure B.39.** Performance plots for well 199-N-154 (P-7-R)

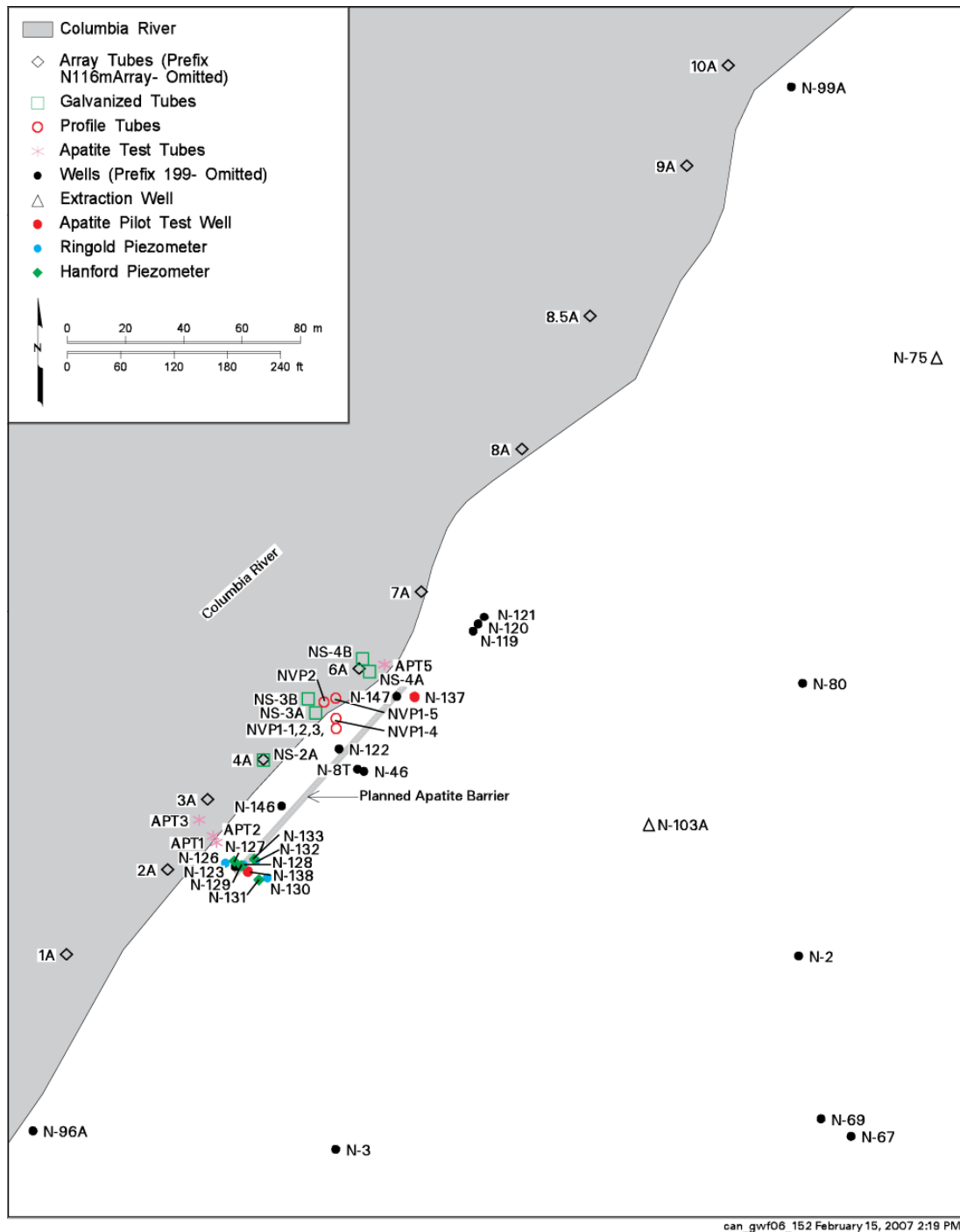


**Figure B.40.** Performance plots for well 199-N-155 (P-6-H)



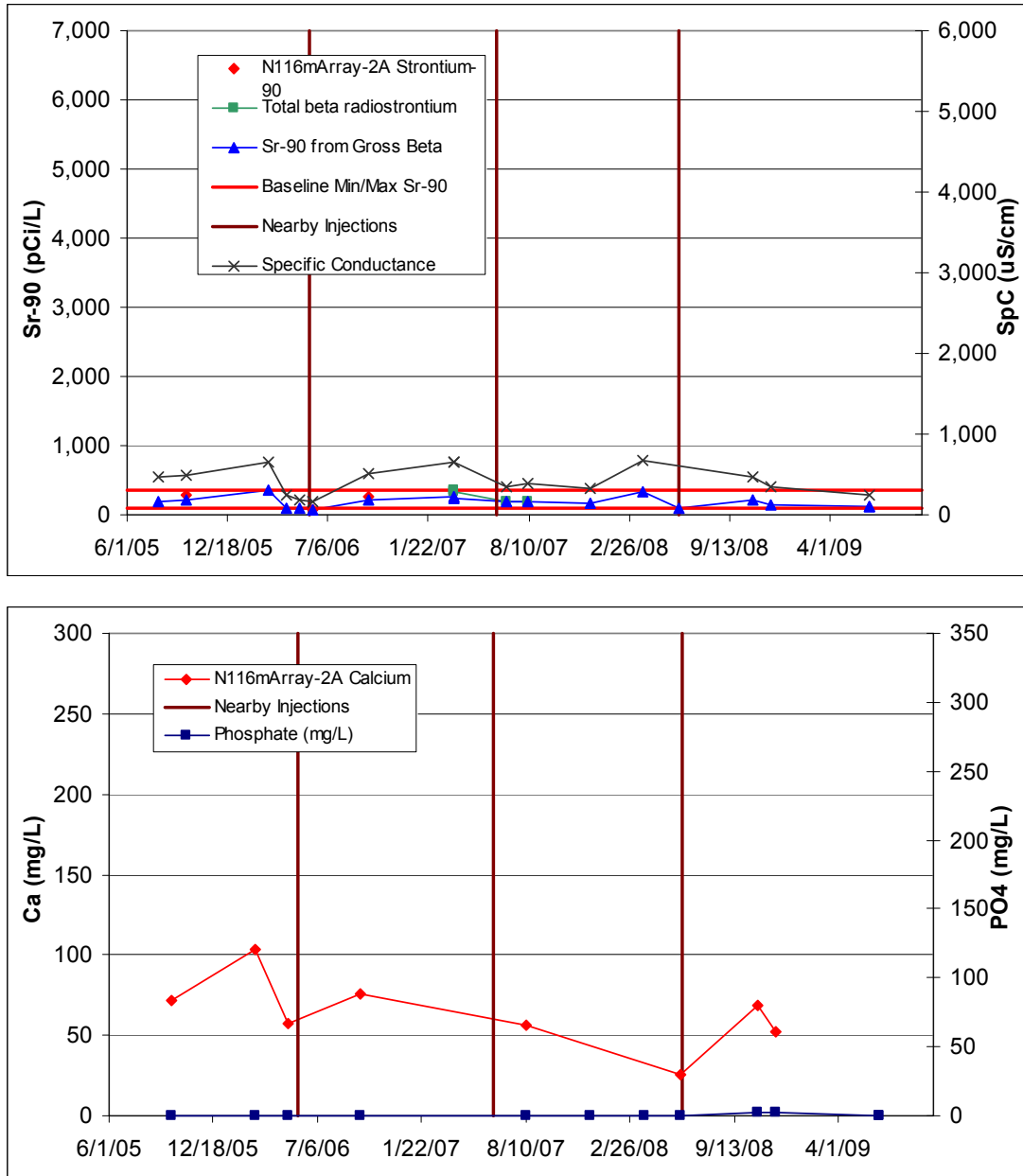
**Figure B.41.** Performance plots for well 199-N-156 (P-5-R)

## B.5 Aquifer Tubes

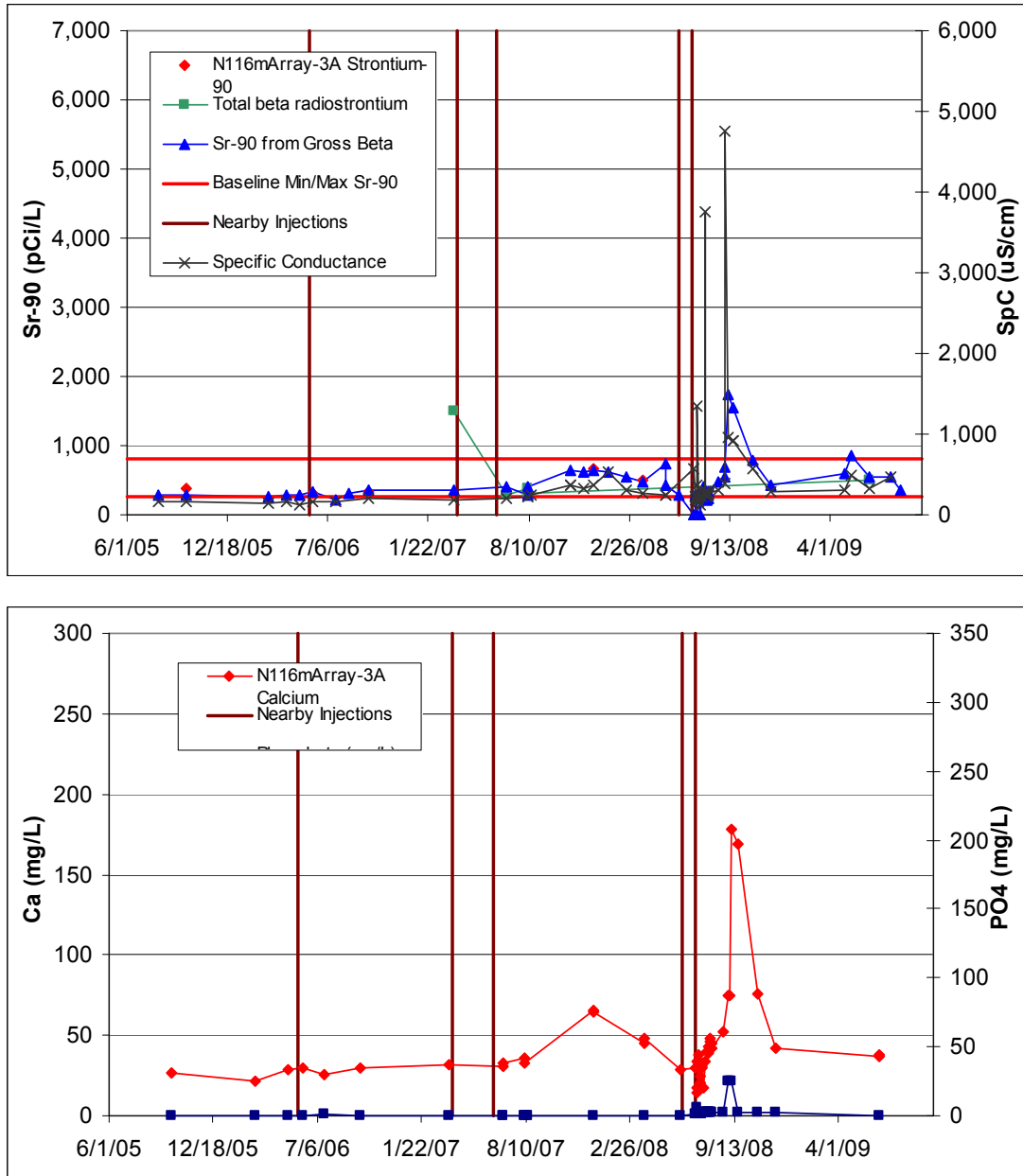


**Figure B.42.** Map of aquifer tube locations

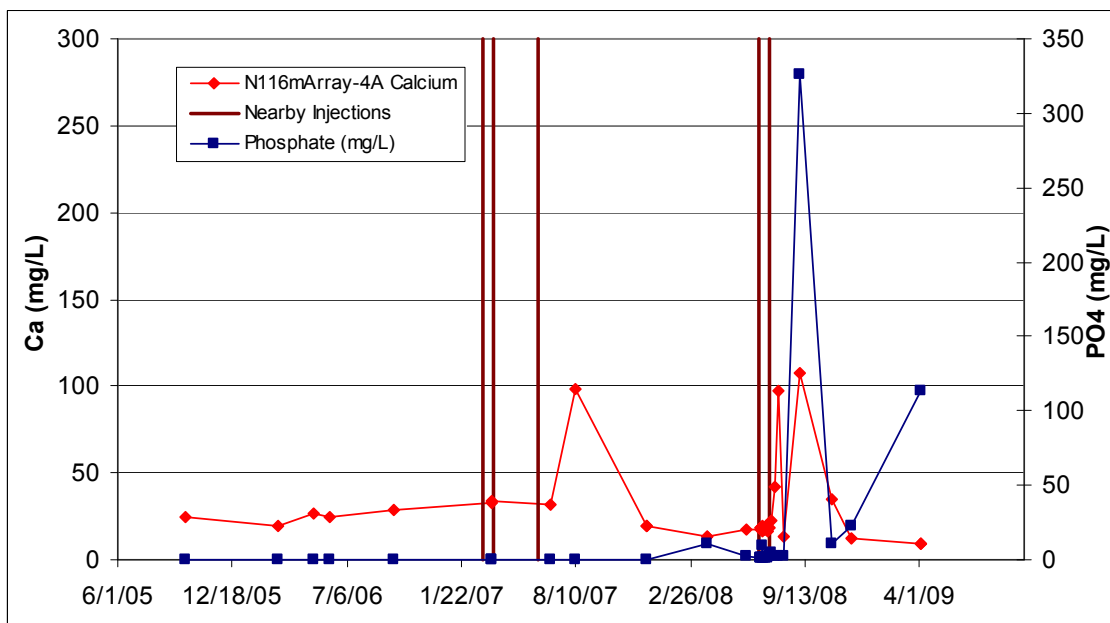
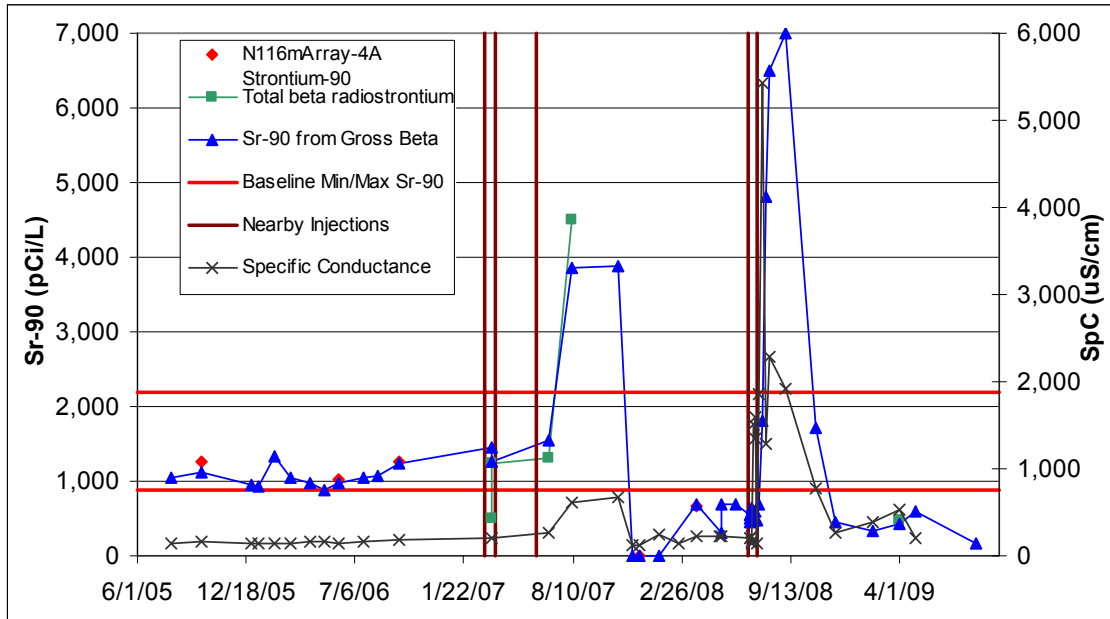




**Figure B.43.** Performance plots for tube 2A



**Figure B.44.** Performance plots for tube 3A



**Figure B.45.** Performance plots for tube 4A

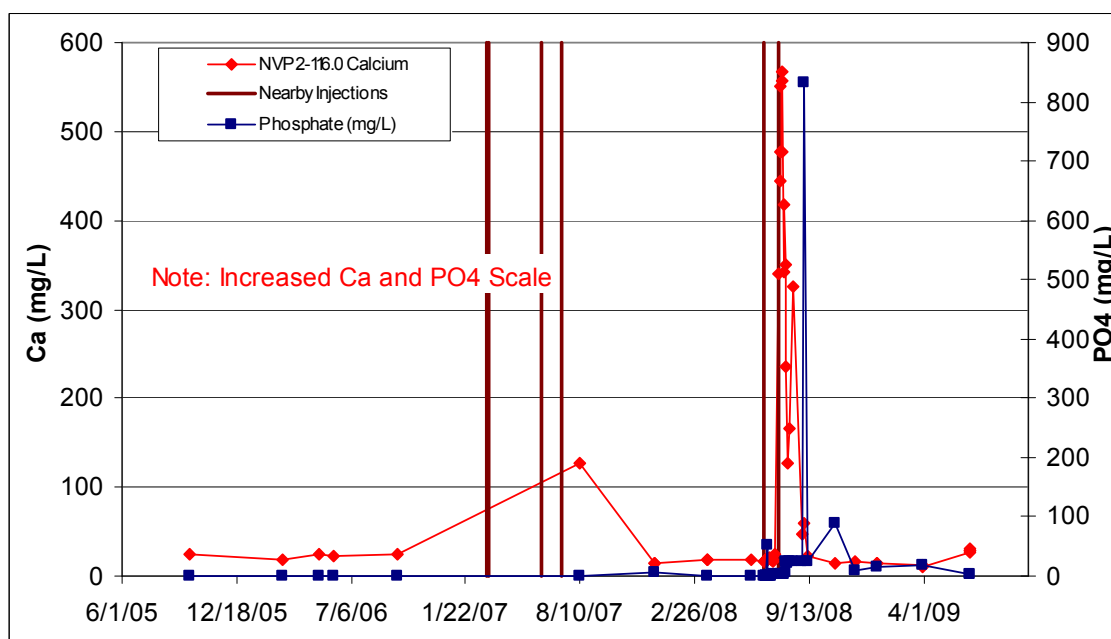
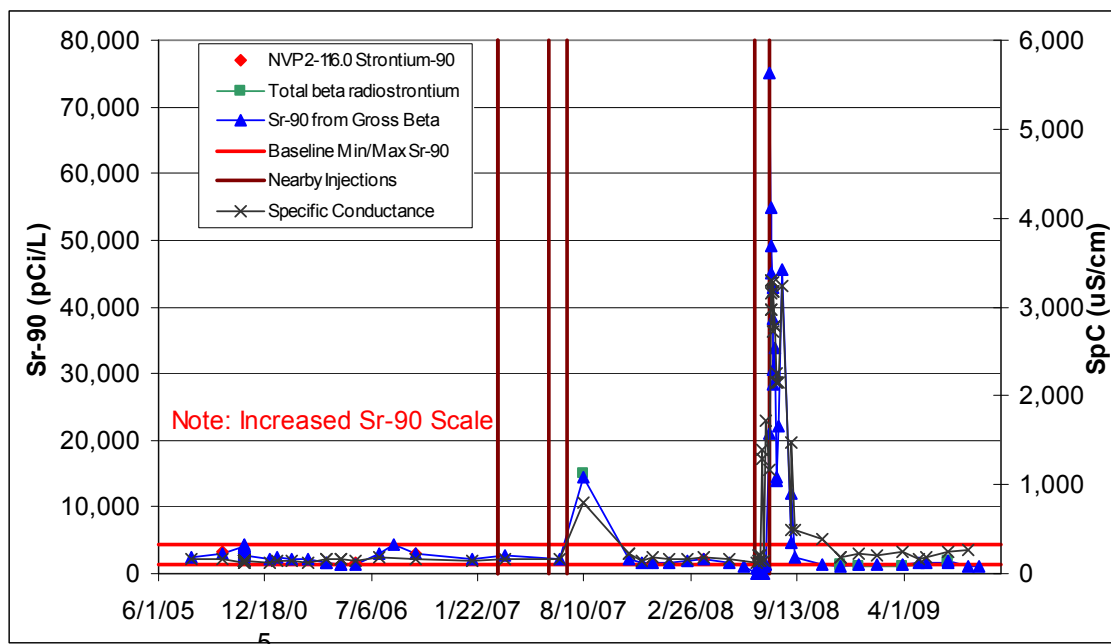
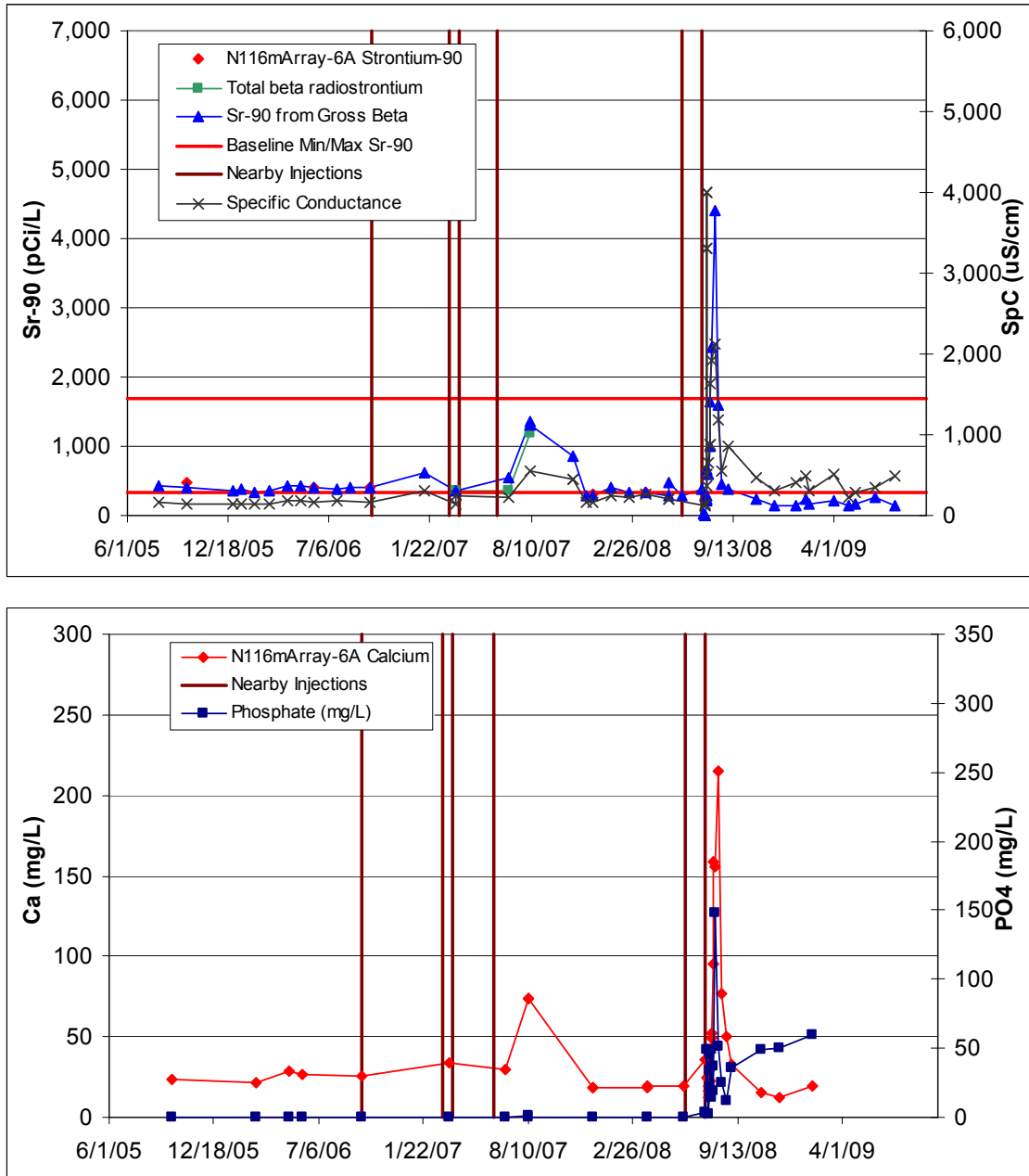
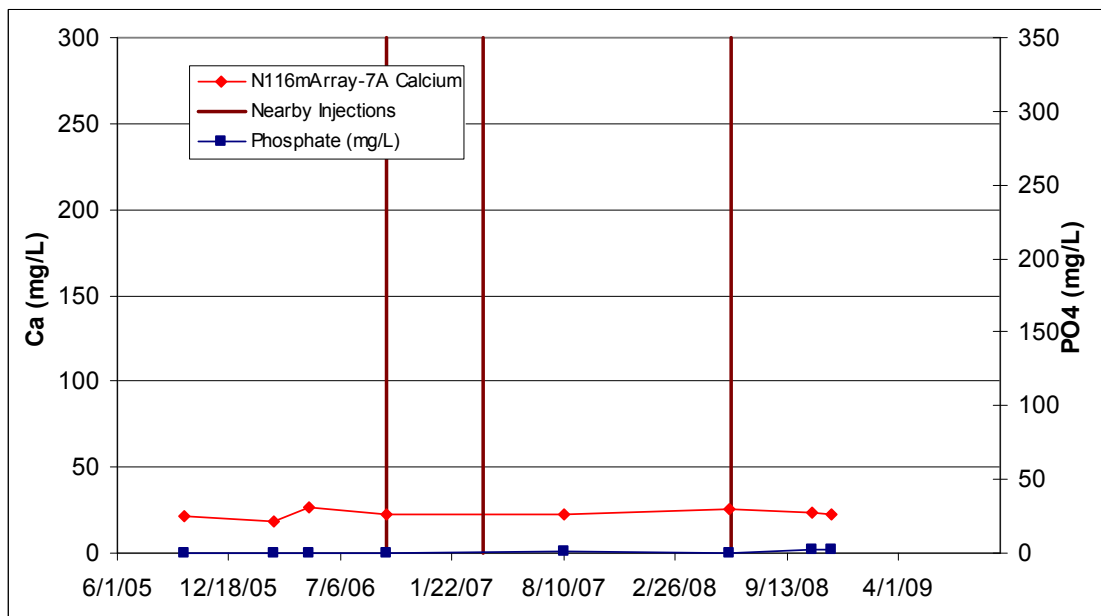
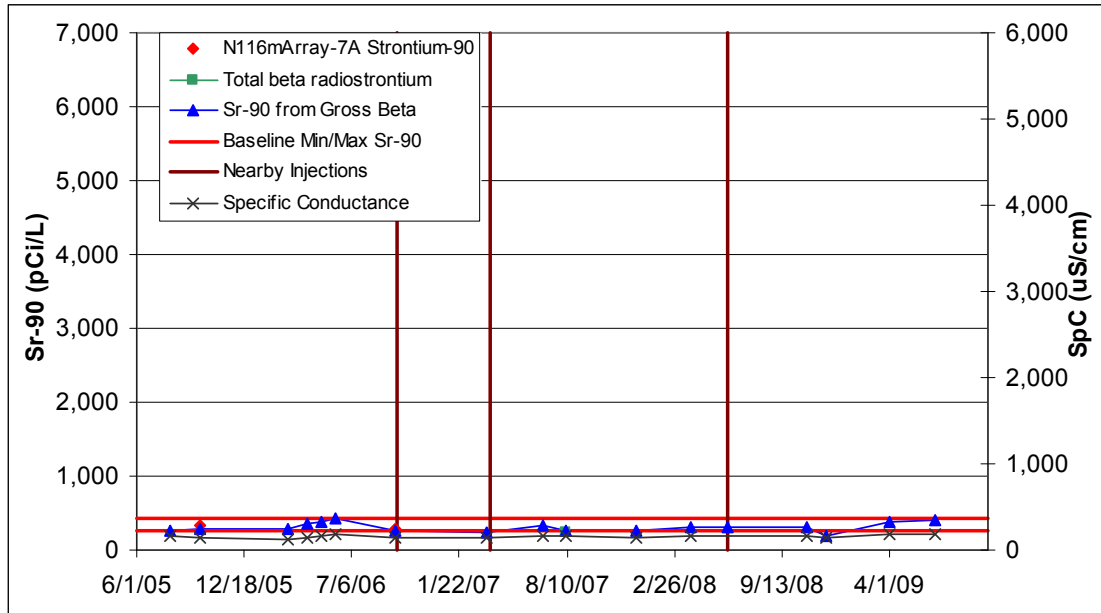


Figure B.46. Performance plots for tube NVP2



**Figure B.47.** Performance plots for tube 6A



**Figure B.48.** Performance plots for tube 7A

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